NTHMP - Mapping & Modeling Benchmarking Workshop: Tsunami Currents

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METU

Department of Civil Engineering and Department of Engineering Sciences.



- Computational tool is NAMI DANCE which is developed by Profs. Andrey Zaytsev, Ahmet Yalciner, Anton Chernov, Efim Pelinovsky and Andrey Kurkin.
- It solves NLSW and also visualize the results by animations for the assessment, understanding and investigation of tsunami generation, propagation and coastal amplification mechanisms. The model is tested and verified in different Benchmark problems

Short History

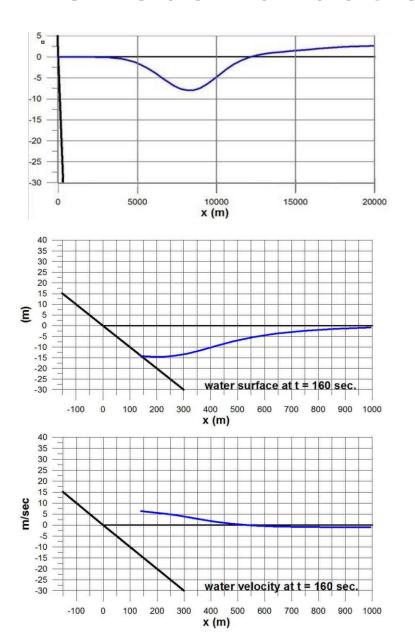
- Based on TUNAMI N2 developed in Tohoku University, Japan
- Distributed by TIME Project of UNESCO
- Coverted to C++ and additional modules added in NAMI DANCE

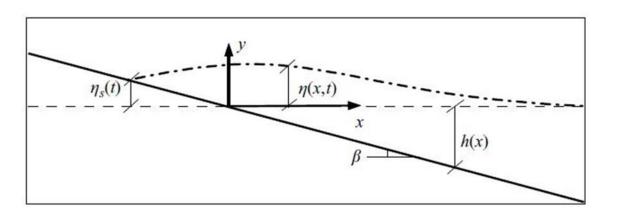
Acknowledgements

UNESCO TIME Project,
Prof. Shuto, Prof. Imamura,
Costas Synolakis, Emile Okal, Efim Pelinovsky

- Catalina Workshop of Long Wave Runup Models
- IAEA Benchmarking of Tsunami Numerical Models

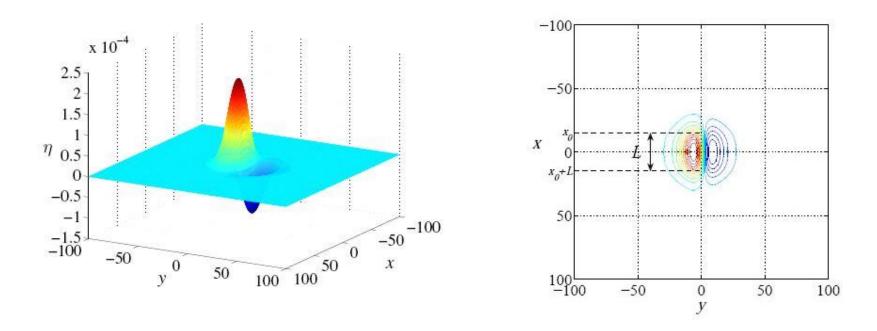
- Benchmark Problems
- Analytical Data
- Experimental Data
- Field Data



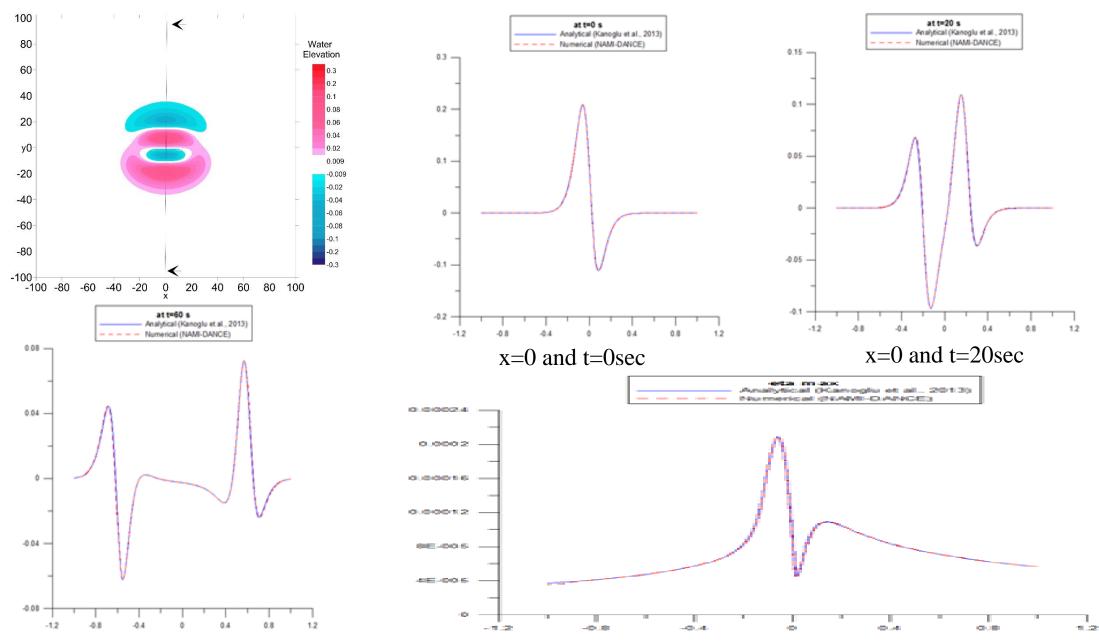


[3-6] Kânoglu, U., (2007): "Theoretical solution of the wave runup on 1/10 sloping beach", Joint Workshop of Benchmark Problems, Numerical Models, Inundation Maps and Test Sites in EC funded TRANSFER Project, held in Fethiye Turkey on June 12-14, 2007.

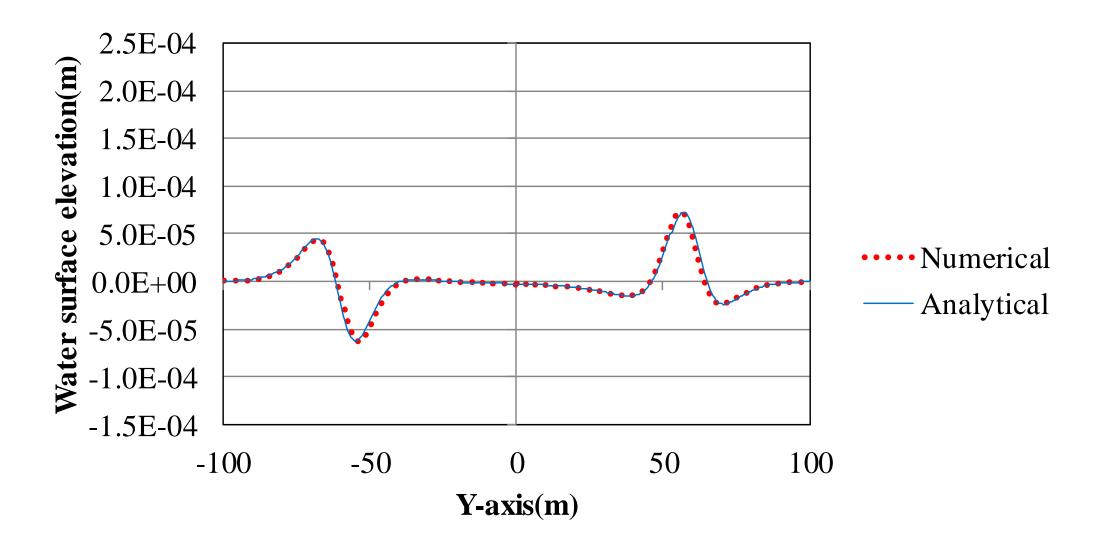
ANALYTICAL BENCHMARK PROBLEM (FOCUSING OF LONG WAVES)



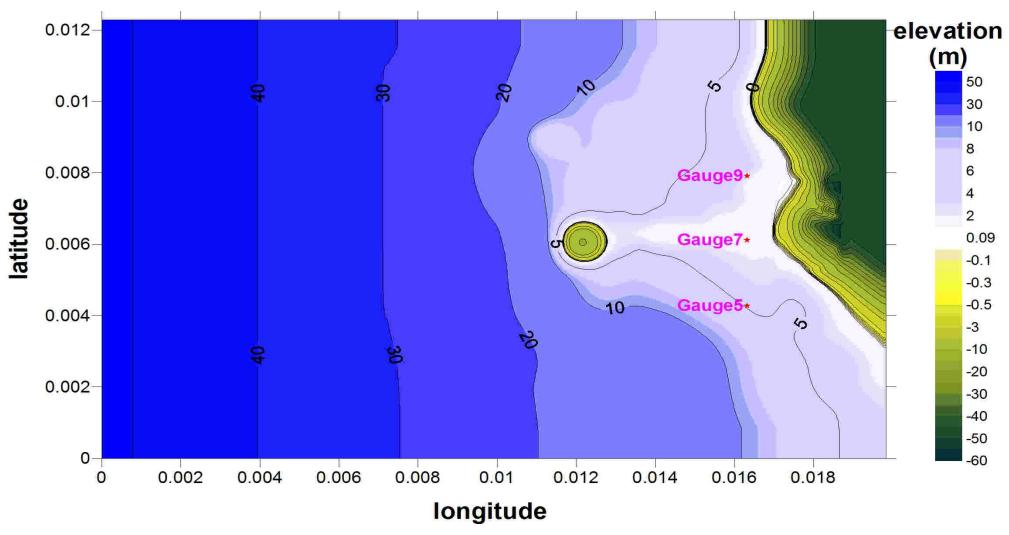
Kânoglu, U., Titov. V., Aydın B., Moore C., Stefanakis S. T., Zhou H., Spillane M., Synolakis C. E.,(2013): "Focusing of long waves with finite crest over constant depth" Proc. R. Soc. A. 2013 469 2153 20130015; doi:10.1098/rspa.2013.0015 (published 27 February 2013) 1471-2946



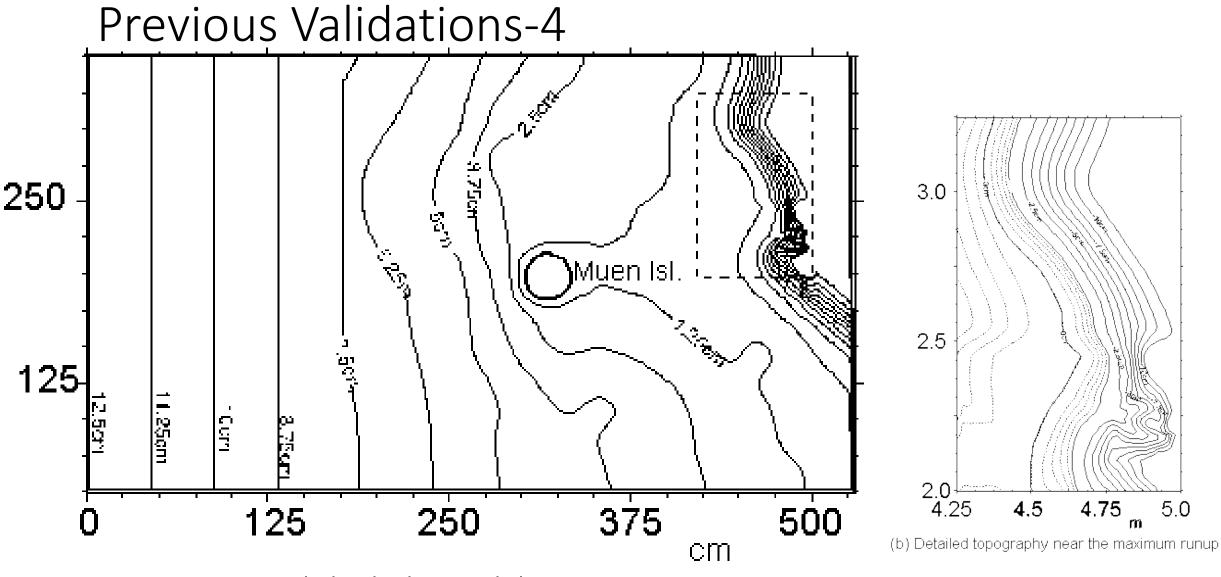
Cross section of water surface elevation of analytical and numerical results Comparison of maximum water elevations of analytical and numerical results



x = 0 and t = 60 sec FIG. I-10Cross section of water surface elevation for analytical and numerical results

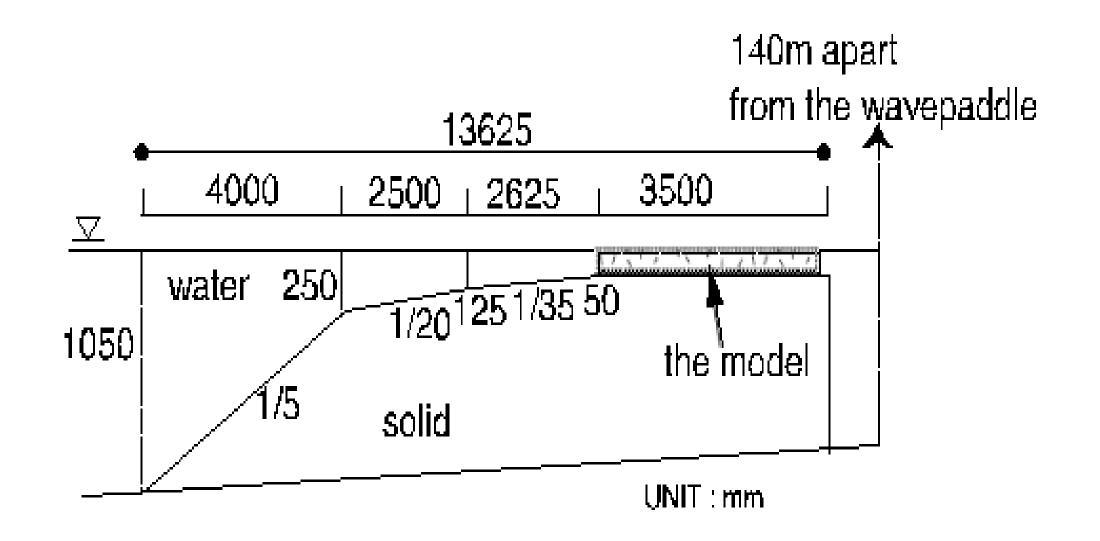


Matsuyama, M., and H. Tanaka (2001), An experimental study of the highest run-up height in the 1993 Hokkaido Nansei-oki earthquake tsunami, U.S. National Tsunami Hazard Mitigation Program Review and International Tsunami Symposium (ITS), Seattle, Washington 7-10 August 2001. U.S. National Tsunami Hazard Mitigation Program, 7(21), 879-889.

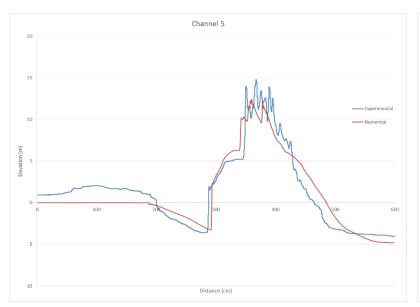


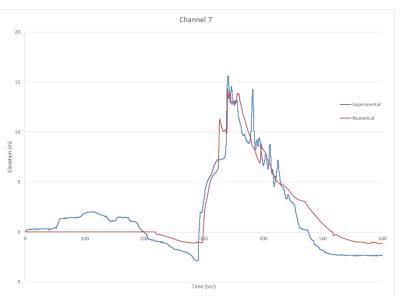
(a) whole model area

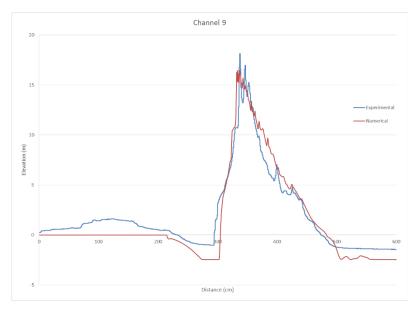
Bathymetry and topography near the runup area



Comparison of the measured and computed water elevations







at channel 5

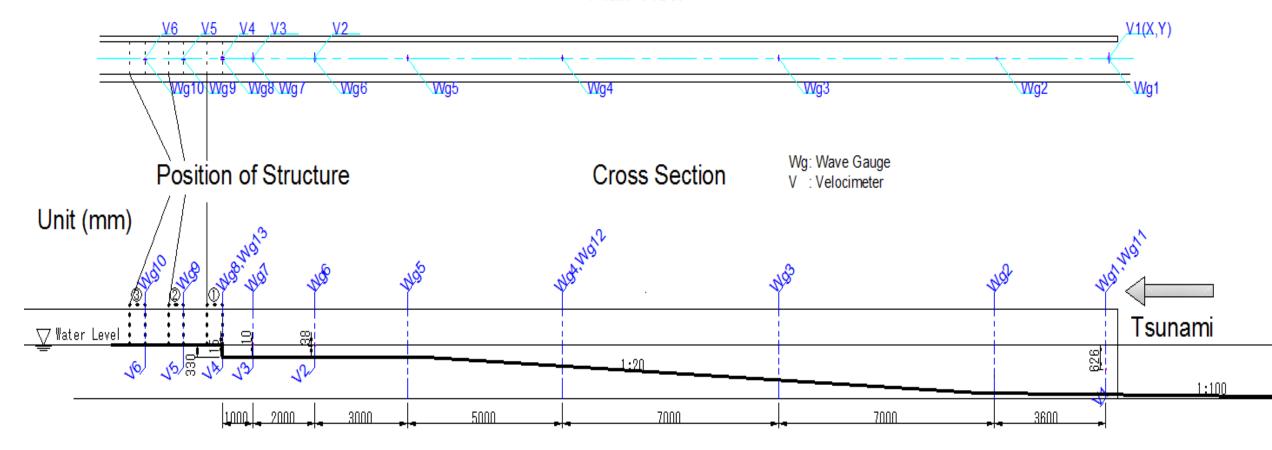
at channel 7

at channel 9

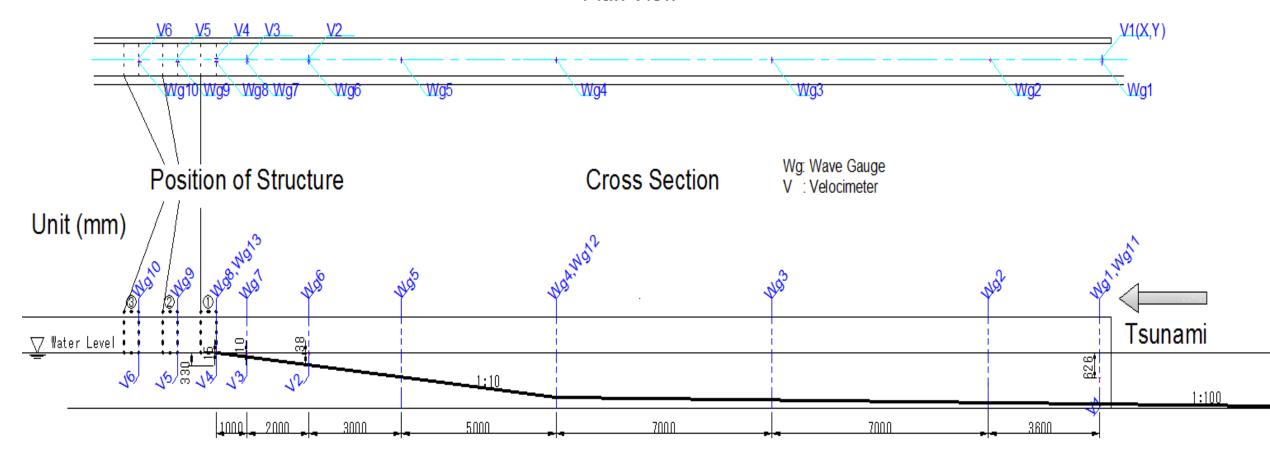
PARI EXPERIMENTS BY DR. ARIKAWA

Port and Airport Research Institute (PARI, Japan) have performed a series of physical model experiments[4-2]. In the experiments the solitary wave climb on different slopes and its impact on vertical wall are tested. Four different channel bottom slopes (near shore slope) are used as i) horizontal, 1:00, ii) 1:10, iii) 1:20, iv) 1:40in front of the block nearest shore line. At the toe of the near shore slope the channel bottom continues with 1:100 slope. In the experiments of each slope, two different heights (large and small) of solitary waves are used. Width of the design structure is 0.80 m in the model. Three identical blocks of wall are located in the shore with 0.50 m width and 1.00 m height.

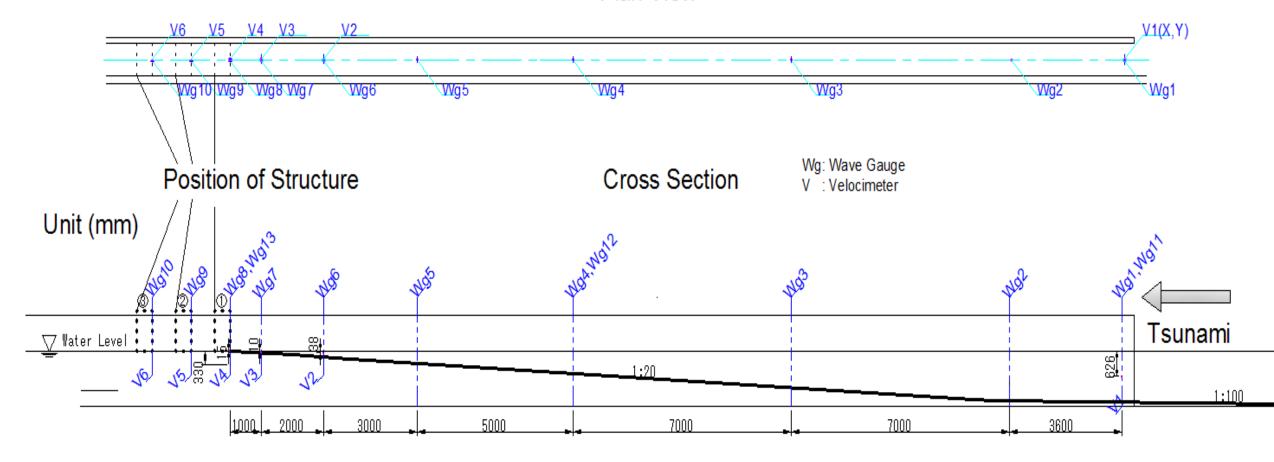
Plan View



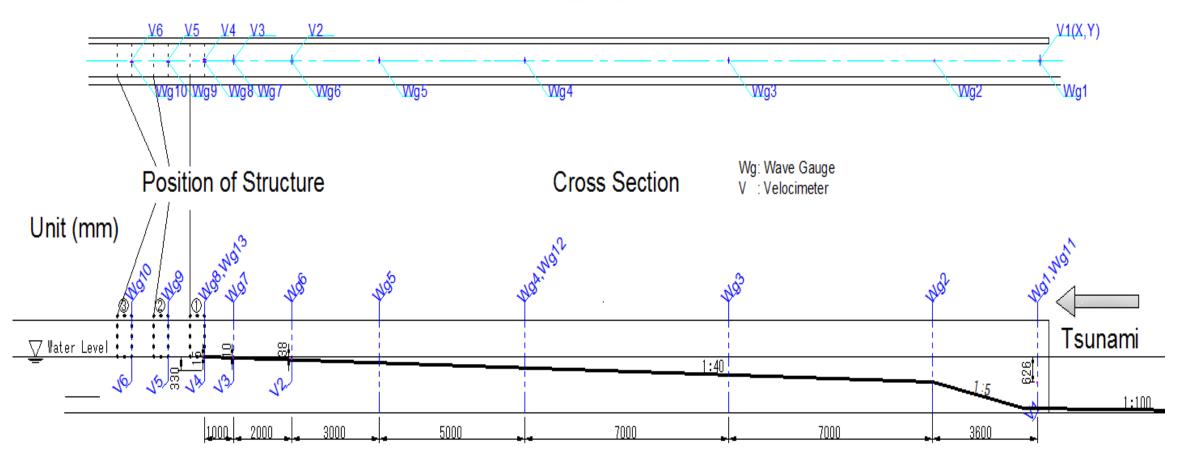
Plan View

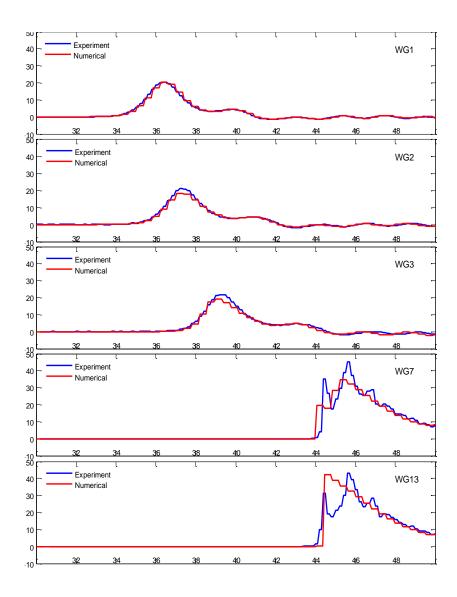


Plan View

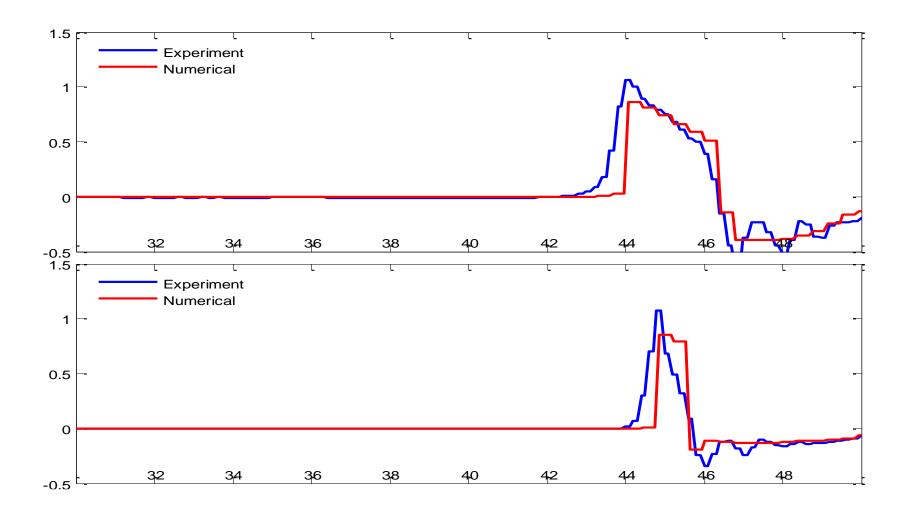


Plan View

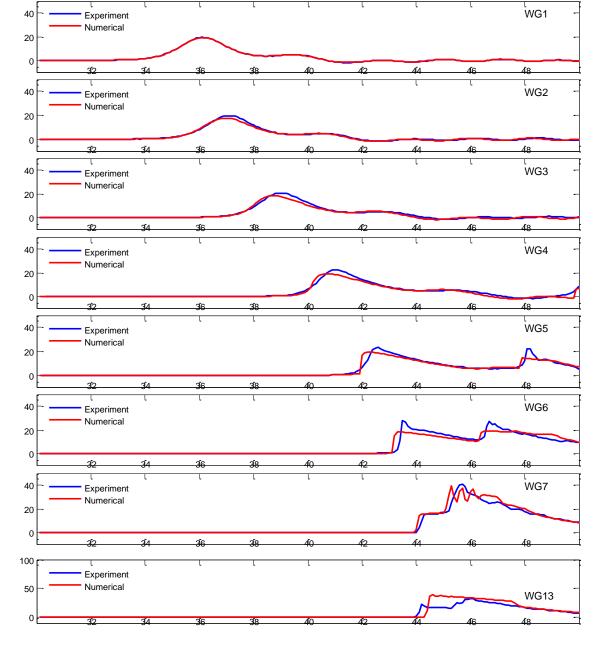




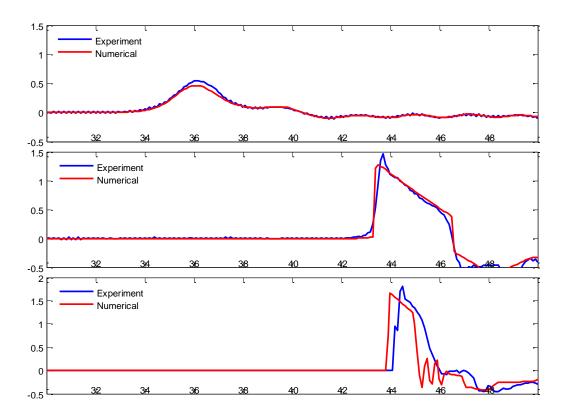
Comparison of the measured and computed water elevations for the front slope of 1-00 horizontal with large input wave at channels WG1, WG2, WG3, WG7 and WG13



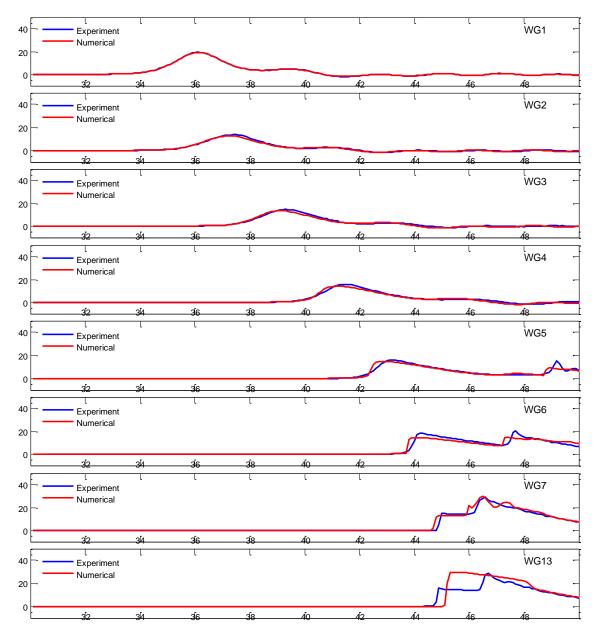
Comparison of the measured and computed water velocity in wave direction for the front slope of 1-00 horizontal with small input wave at channels V2 (top) and V3 (bottom)



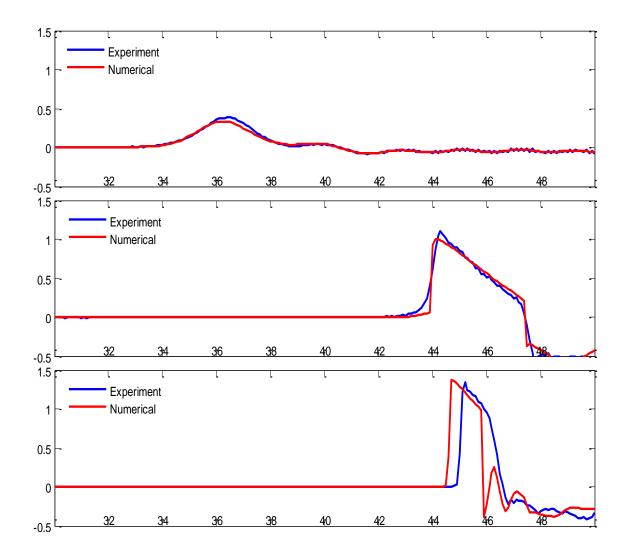
Comparison of the measured and computed water elevations for the front slope of 1-20 with the large input wave at channels WG1, WG2, WG3, WG4, WG5, WG6, WG7 and WG13



Comparison of the measured and computed water velocity in wave direction for the front slope of 1-20 with large input wave at channels V1 (top), V2 (centre), and V3 (bottom)



parison of the measured and computed water elevations for the front slope of 1-20 with the small input wave at channels WG1, WG2, WG3, WG4, WG5, WG6, WG7 and WG13



Comparison of the measured and computed water velocity in wave direction for the front slope of 1-20 with small input wave at channels V1 (top), V2 (centre), and V3 (bottom)



NTHMP - Mapping & Modeling Benchmarking Workshop: Tsunami Currents

Benchmark Problem #1 Shallow Flow Around A Submerged Conical Island With Small Side Slopes

Ahmet Cevdet Yalciner, Andrey Zaytsev, Utku Kanoğlu
Ph. D. Candidate Deniz Velioğlu



Middle East Technical University
Civil Engineering Department

BM#1 – Shallow Flow Around A Submerged Conical Island With Small Side Slopes

While there are many experimental datasets looking at the wake behind a cylinder, there are very few that examine the wake behind a sloping obstacle in the context of shallow flow.

A conical island is placed on a flat bottom, where the water depth is 0.054 m. The side slopes of the conical island are 8 degrees,

The height of the island is 0.049 m and

The diameter at the base of the island is 0.75 m

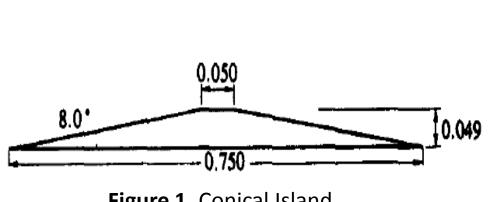


Figure 1. Conical Island

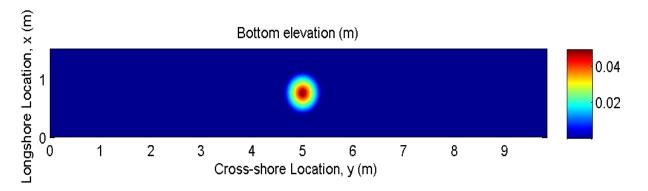


Figure 2. Bathymetry

BM#1 – Shallow Flow Around A Submerged Conical Island With Small Side Slopes

In this study, horizontal velocity components located at two different locations behind the island are compared. A plot of the locations is shown in Figure 3. Point (1) is located 1.02 m behind the center of the island. Point (2) is located at the same x-location as Point (1), but 0.27 m offset in the positive (y) direction

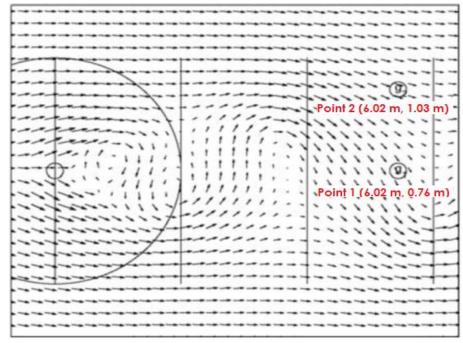


Figure 3. Two gauge points located behind the submerged cone

BM#1 – Shallow Flow Around A Submerged Conical Island With Small Side Slopes

Input Data:

The steady discharge velocity is U = 0.115 m/s

Water depth is h = 0.054 m

 $\Delta x = 0.01 \text{ m } \& \Delta t = 0.001 \text{ sec}$

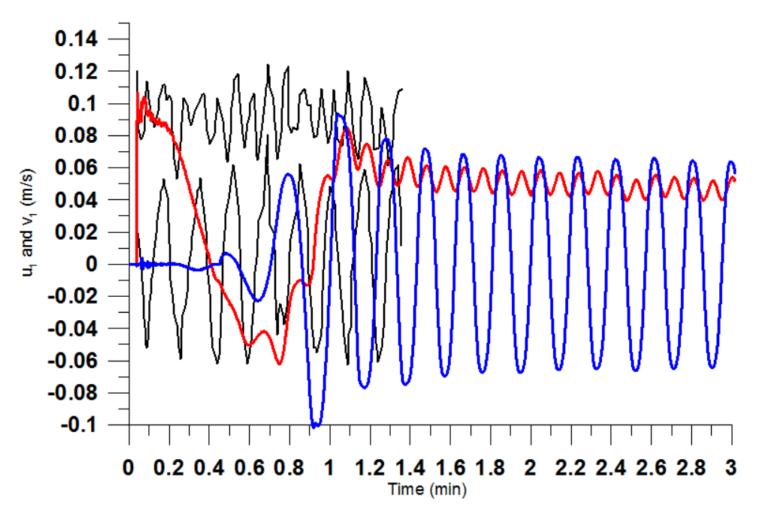
Manning's Roughness Coefficient, n= 0.01

Simulation Duration: 3 minutes

Figure 4. Bathymetry used in the simulations

x = 5.0 m

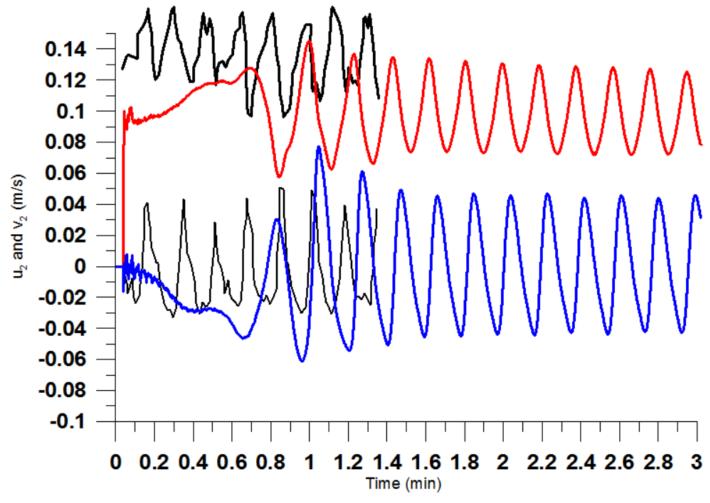
COMPARISON OF RESULTS (Gauge Point 1):



Point 1: U (top) and V (bottom) Experimental Data: Black Line

Numerical Results: Red Line (U)/Blue Line (V)

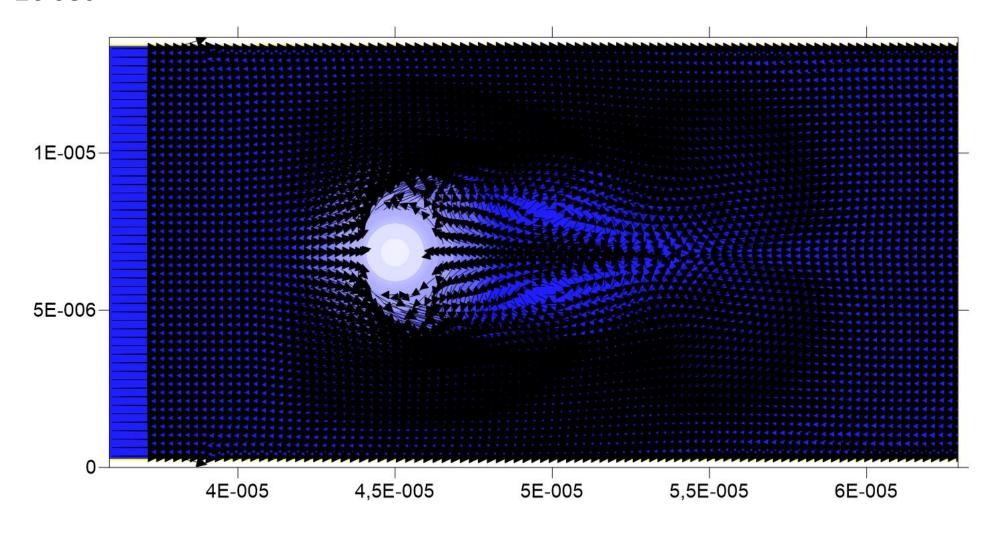
COMPARISON OF RESULTS (Gauge Point 2):



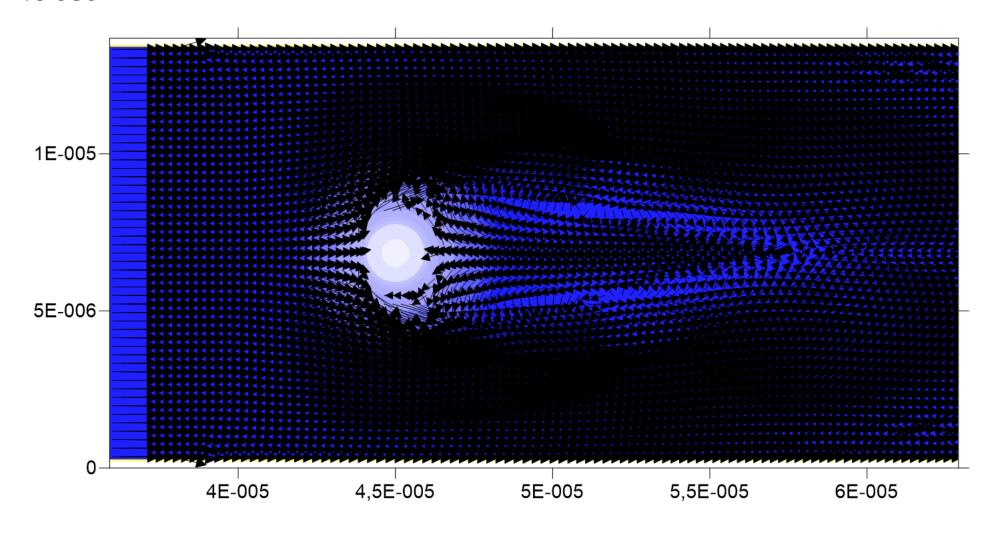
Point 2: U (top) and V (bottom) Experimental Data: Black Line

Numerical Results: Red Line (U)/Blue Line (V)

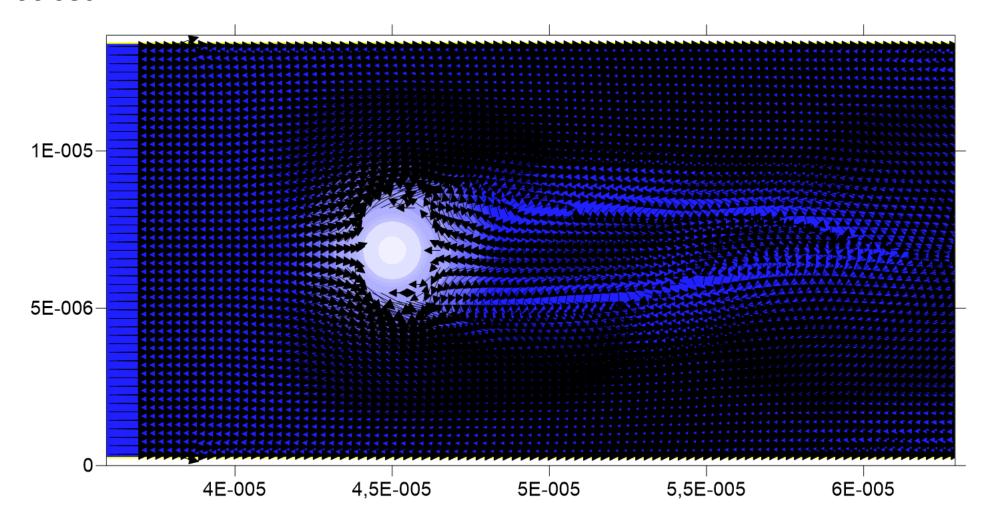
t = 20 sec



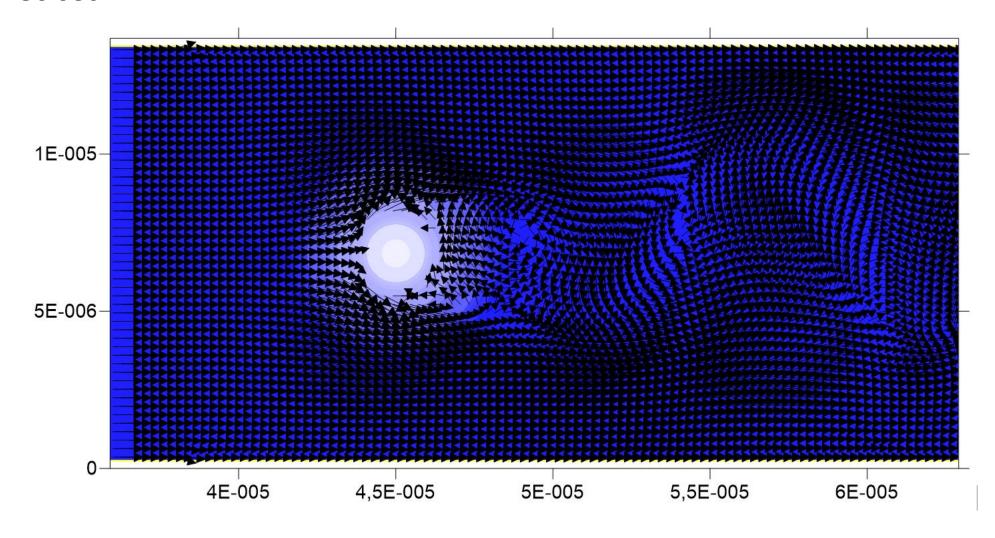
t = 40 sec



t = 60 sec

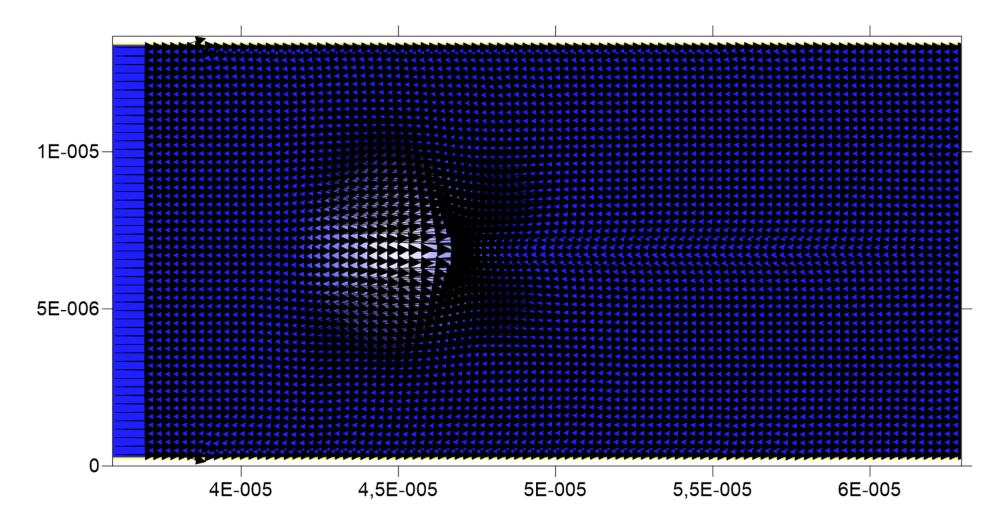


t = 80 sec

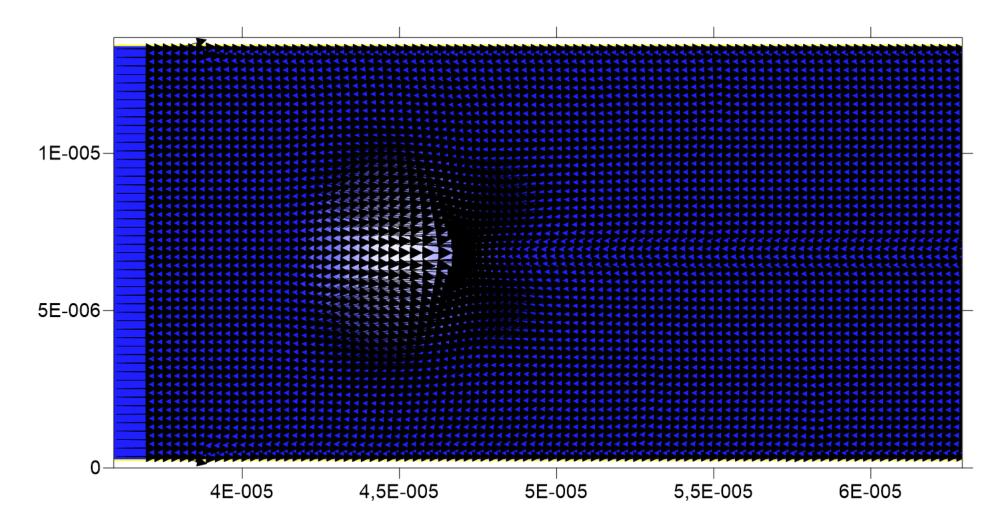


VELOCITY VECTORS AT DIFFERENT TIME INTERVALS (Configuration 3)

t = 20 sec

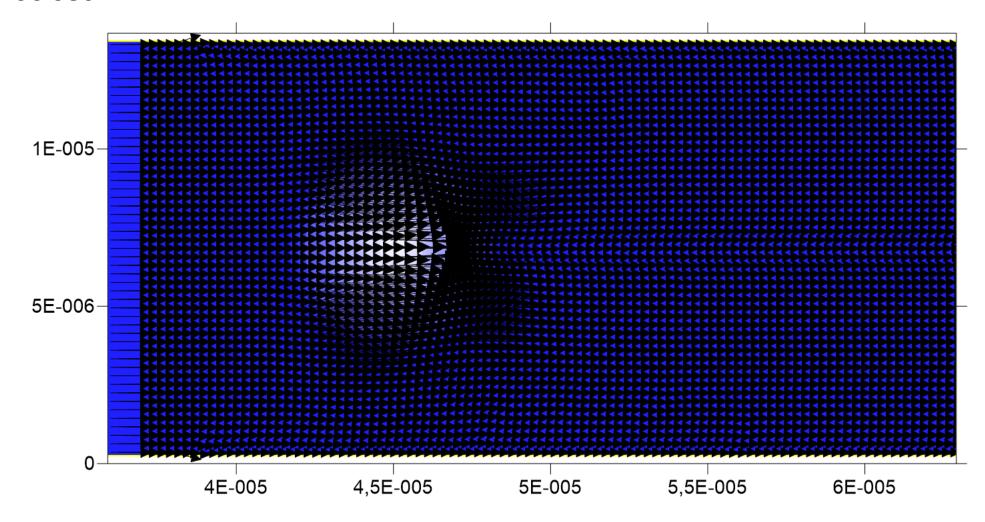


t = 40 sec



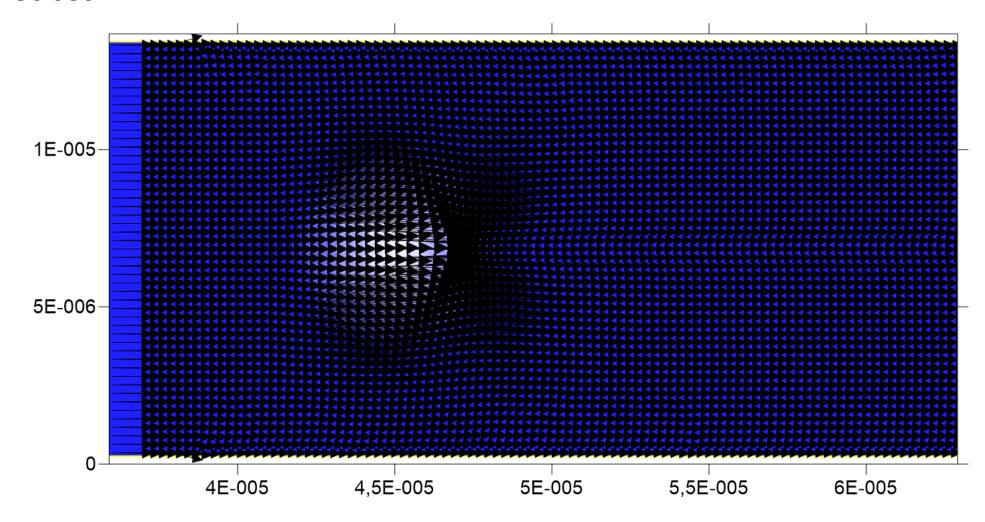
VELOCITY VECTORS AT DIFFERENT TIME INTERVALS

t = 60 sec



VELOCITY VECTORS AT DIFFERENT TIME INTERVALS

t = 80 sec







NTHMP - Mapping & Modeling Benchmarking Workshop: Tsunami Currents

Benchmark Problem #2 Japan 2011 tsunami in Hilo Harbor, Hawaii

Ahmet C. Yalciner, Andrey Zaytsev, Utku Kanoglu Research Assistant Gozde Guney Dogan

METU, Department of Civil Engineering and Department of Engineering Sciences

BM#2 – Japan 2011 Tsunami in Hilo Harbor, Hawaii

- Several simulations for a comparison of shallow water, tsunami currents aiming to understand the level of precision that can be expected from a model about modelling currents on real bathymetry and to see the convergence of a model with respect to speed predictions and model resolution.
- The results obtained from these simulation studies using NAMI DANCE and the comparisons with the actual data for free surface elevation and current speeds in E-W and N-S directions are provided.

BM#2 – Japan 2011 Tsunami in Hilo Harbor, Hawaii

> 7 Different Configurations

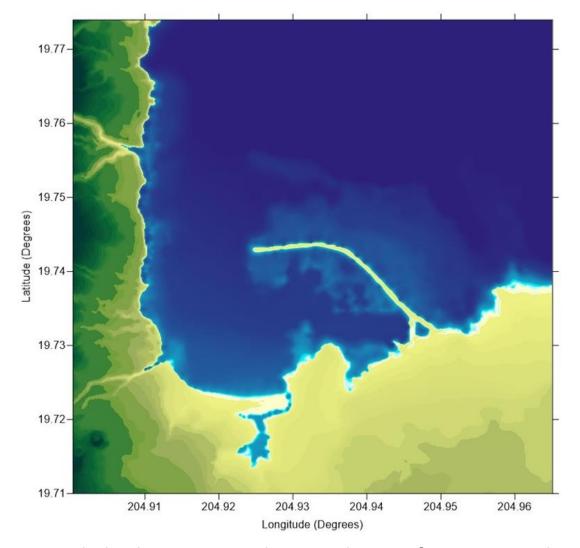
	Dx	Incident wave is inputted at:	Input Location	Manning's Roughness Coefficient
İ	5m resolution	X = from 204.90028 to 204.96509, Y = 19.772115	Upper Grid Boundary	n = 0.015
	10m resolution	X = from 204.90028 to 204.96509, Y= 19.748311	Northern Part of Domain	n = 0
	10m resolution	X = from 204.90028 to 204.96509, Y = 19.757315	Northern Part of Domain—Control Point Level	n = 0
	10m resolution	X = from 204.90028 to 204.96509, Y = 19.773118	Upper Grid Boundary	n = 0
	10m resolution	X = from 204.90028 to 204.96509, Y = 19.773597	Upper Grid Boundary	n = 0.015
	20m resolution	X = from 204.90028 to 204.96509, Y = 19.772064	Upper Grid Boundary	n = 0.015
	20m resolution	X = from 204.90028 to 204.96509, Y = 19.772064	Upper Grid Boundary	n = 0.025

BM#2

BM#2 – Japan 2011 Tsunami in Hilo Harbor, Hawaii

Bathymetry

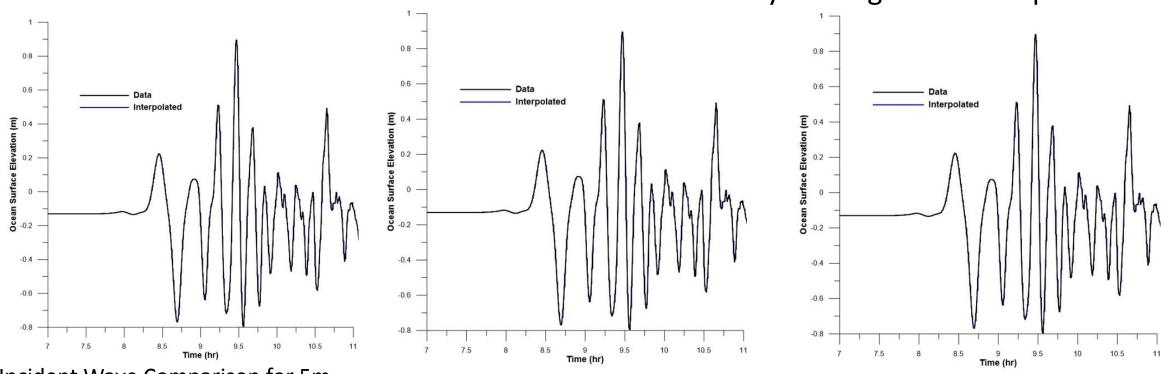
- Bathymetry data is provided (lon,lat) on a 1/3 arcsec grid.
- However, the problem has a flattening of the bathymetry at a depth of 30 meters. Therefore, in the offshore portion of the bathymetry grid, there are no depths greater than 30 m.
- The data is obtained for 20m (2/3 arcsec, the input bathymetry is de-sampled), 10m (1/3 arcsec) and 5m (1/6 arcsec, bi-linear interpolation is used) resolutions.



Sample bathymetry used in simulations for 10 m resolution

BM#2 – Japan 2011 Tsunami in Hilo Harbor, Hawaii Incident Wave

• Since the incident wave data is given with 30 second (0.5 minute) time intervals, the data is obtained again for 0.125 second intervals for 5m resolution case, 0.25 second intervals for 10m resolution and 0.5 second intervals for 20 resolution by making linear interpolation.

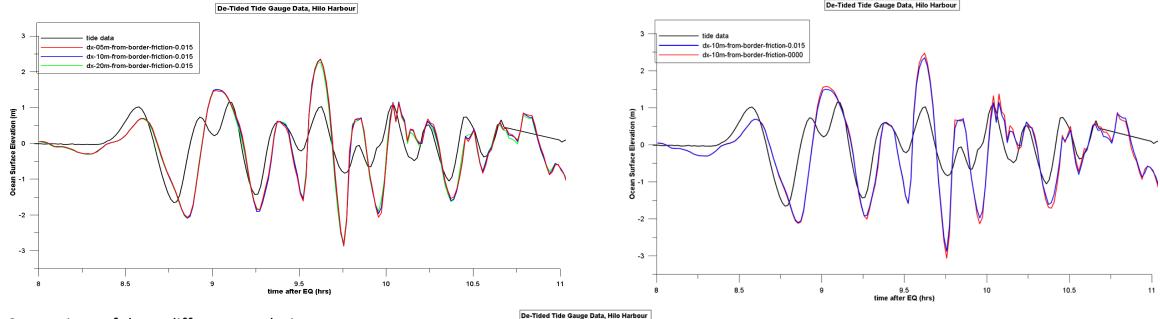


Incident Wave Comparison for 5m resolution (Interpolated data dt = 0.125s)

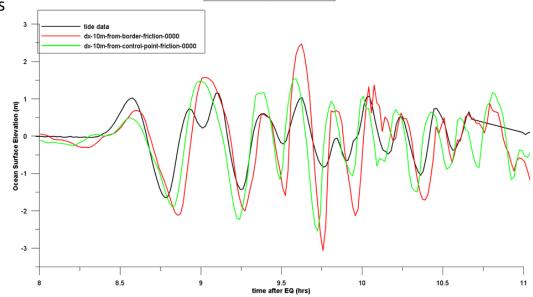
Incident Wave Comparison for 10m resolution (Interpolated data dt = 0.25s)

Incident Wave Comparison for 20m resolution Interpolated data dt = 0.5s

COMPARISON OF RESULTS - De-tided Tide Gauge Data

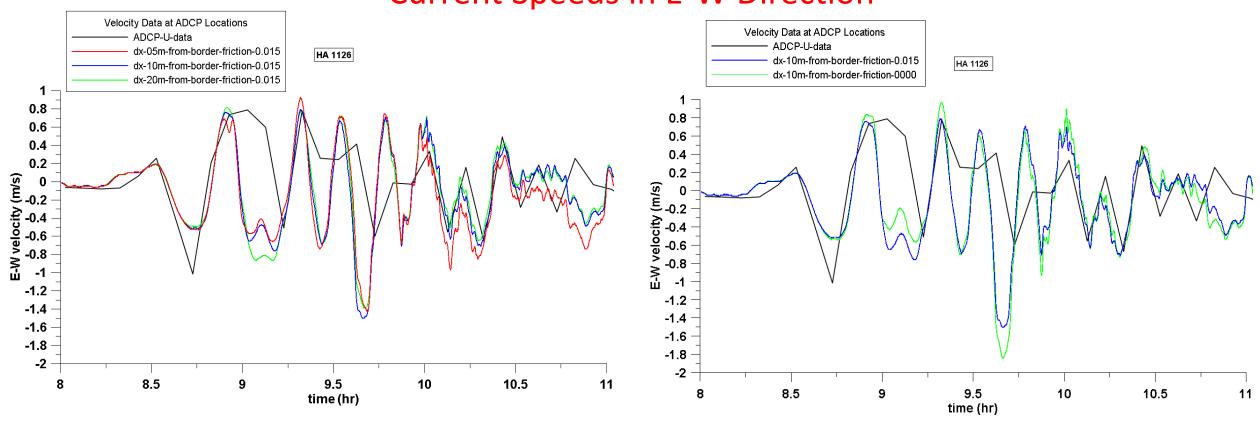


Comparison of three different resolutions for the tide gauge data (n is 0.015)



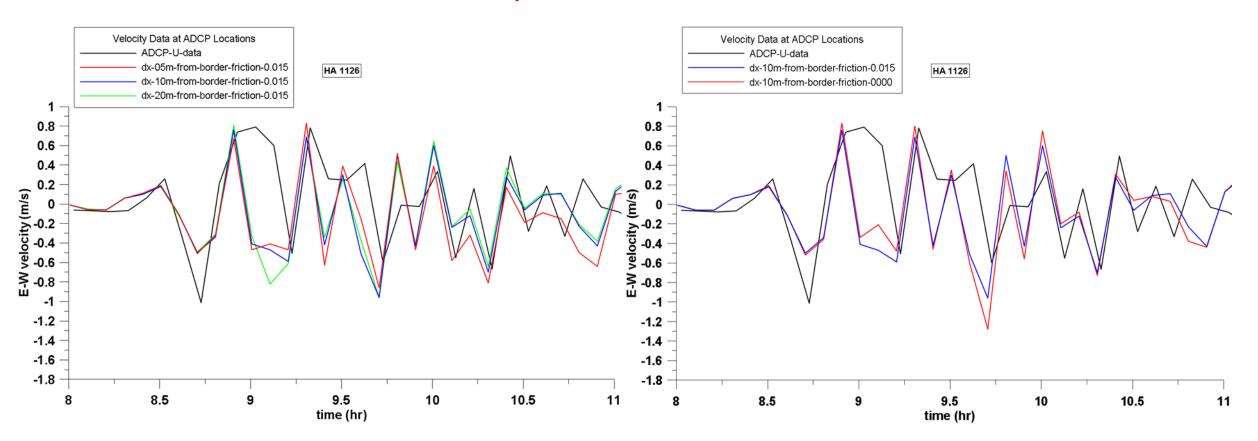
Comparison of Manning's coefficients (0.015 and 0 cases) with 10m resolution for the tide gauge data

Comparison of the input location (from border or control point) of incident wave with 10m resolution



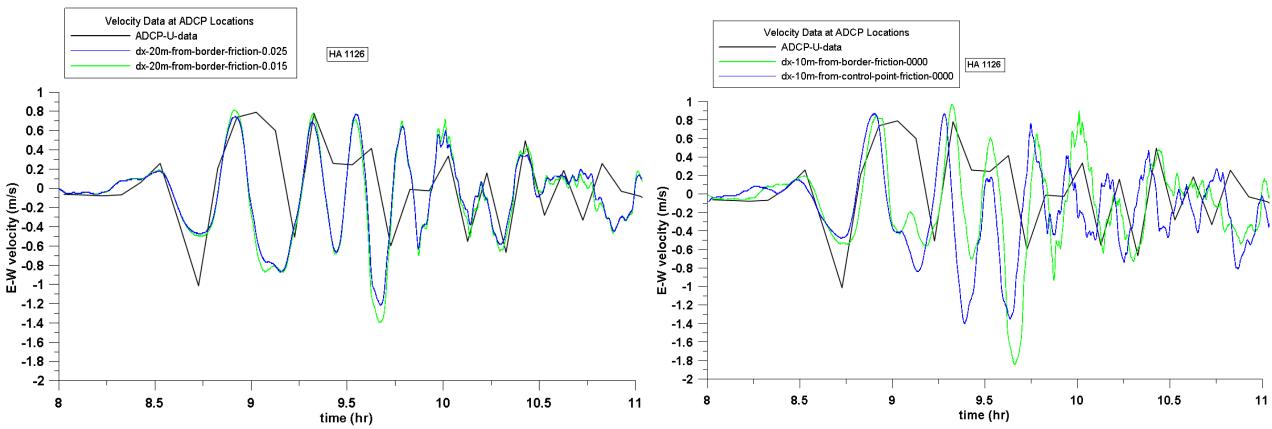
HA1126: Hilo Harbor, Comparisons of the E-W current speeds for three different resolutions (Manning's Coefficient is 0.015)

HA1126: Hilo Harbor, Comparisons of the E-W current speeds for 10m resolution with Manning's coefficient 0 and 0.015



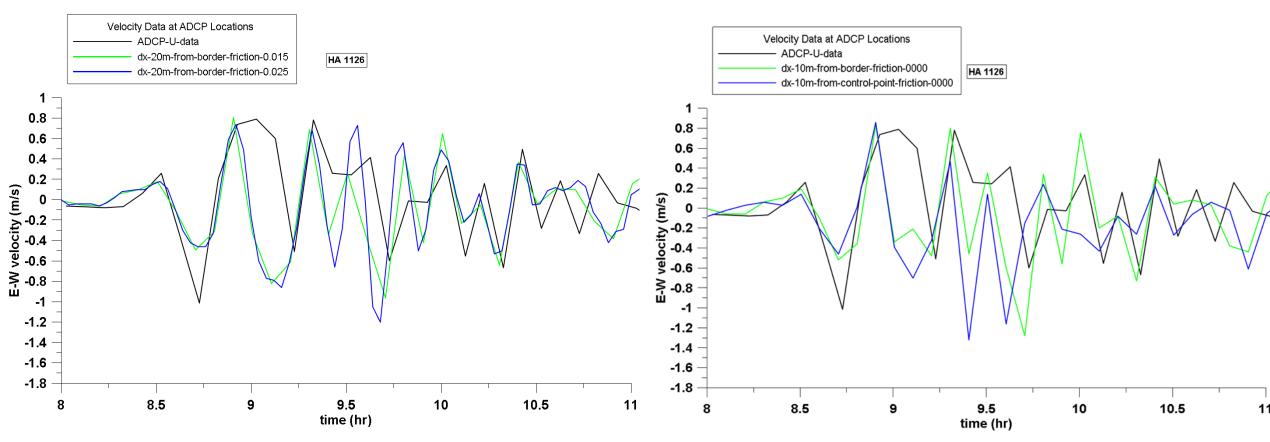
HA1126: Hilo Harbor, Comparisons of the E-W current speeds for three different resolutions (Manning's Coefficient is 0.015)

HA1126: Hilo Harbor, Comparisons of the E-W current speeds for 10m resolution with Manning's coefficient 0 and 0.015



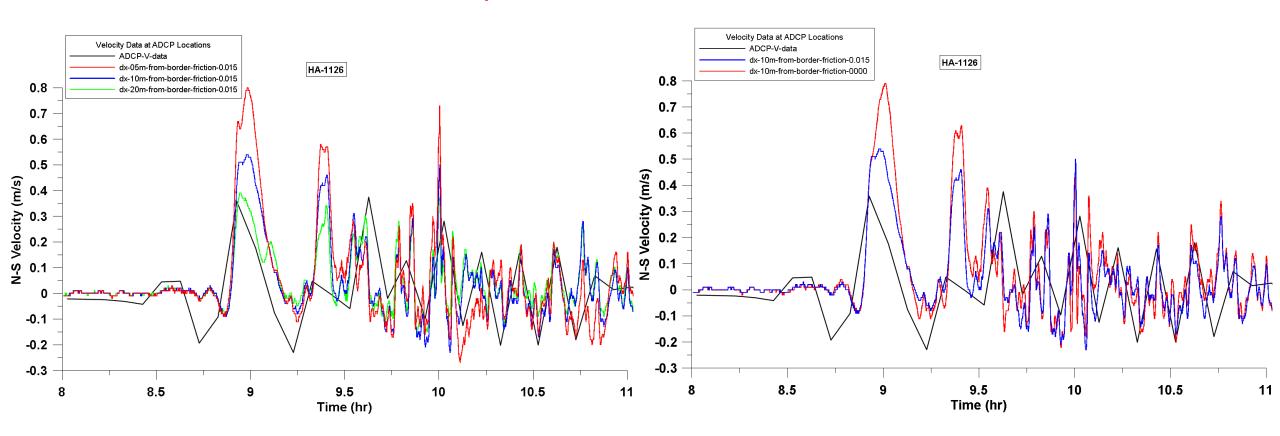
HA1126: Hilo Harbor, Comparisons of the E-W current speeds for 20m resolution with Manning's coefficient 0.015 and 0.025

HA1126: Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in E-W direction



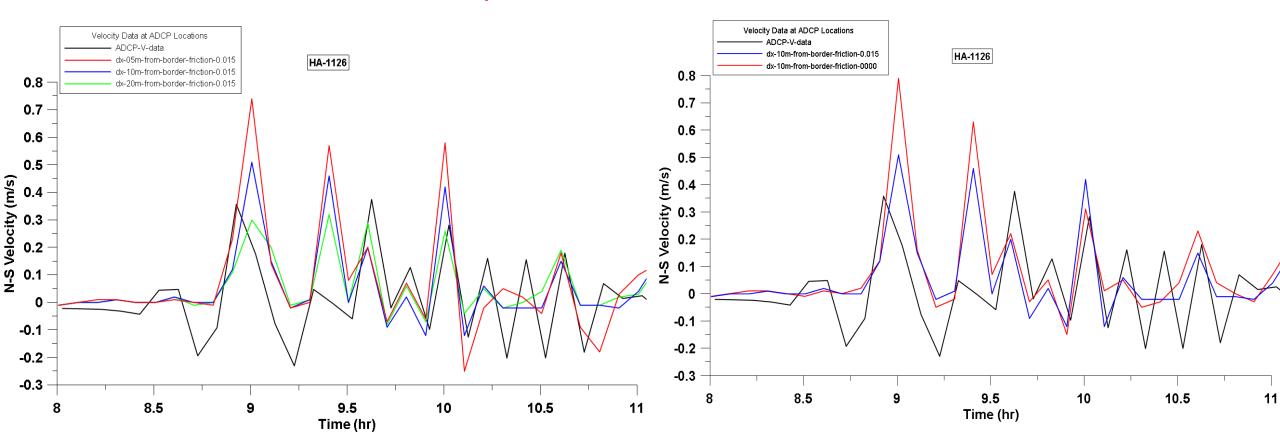
HA1126: Hilo Harbor, Comparisons of the E-W current speeds for 20m resolution with Manning's coefficient 0.015 and 0.025

HA1126: Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in E-W direction



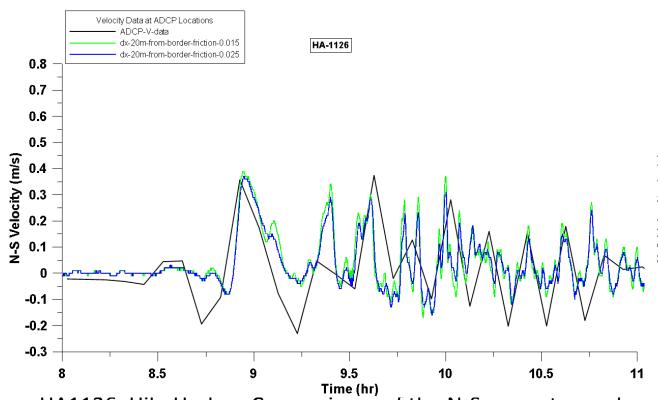
HA1126: Hilo Harbor, Comparisons of the N-S current speeds for three different resolutions (Manning's Coefficient is 0.015)

HA1126: Hilo Harbor, Comparisons of the N-S current speeds for 10m resolution with Manning's coefficients 0 and 0.015

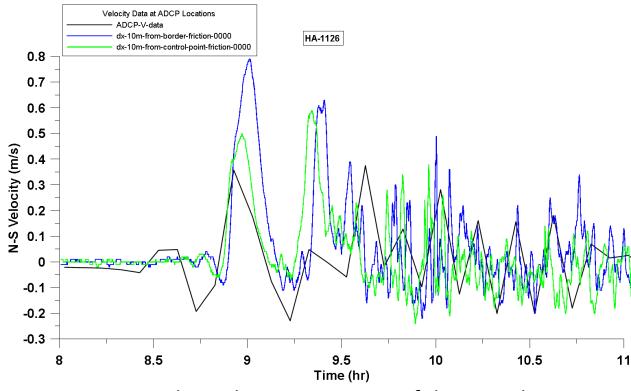


HA1126: Hilo Harbor, Comparisons of the N-S current speeds for three different resolutions (Manning's Coefficient is 0.015)

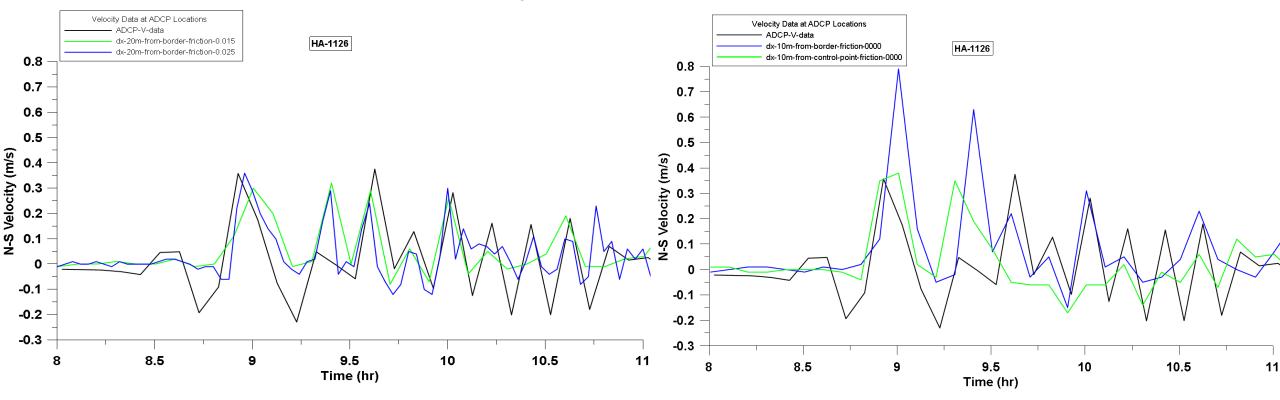
HA1126: Hilo Harbor, Comparisons of the N-S current speeds for 10m resolution with Manning's coefficients 0 and 0.015



HA1126: Hilo Harbor, Comparisons of the N-S current speeds for 20m resolution with Manning's coefficients 0.015 and 0.025



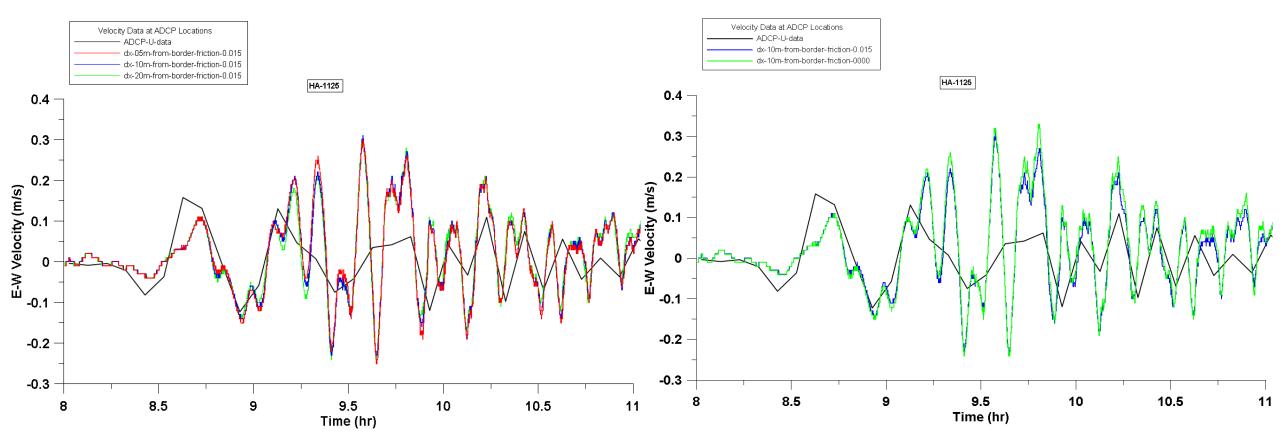
HA1126: Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in N-S direction



HA1126: Hilo Harbor, Comparisons of the N-S current speeds for 20m resolution with Manning's coefficients 0.015 and 0.025

HA1126: Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in N-S direction

Current Speeds in E-W Direction

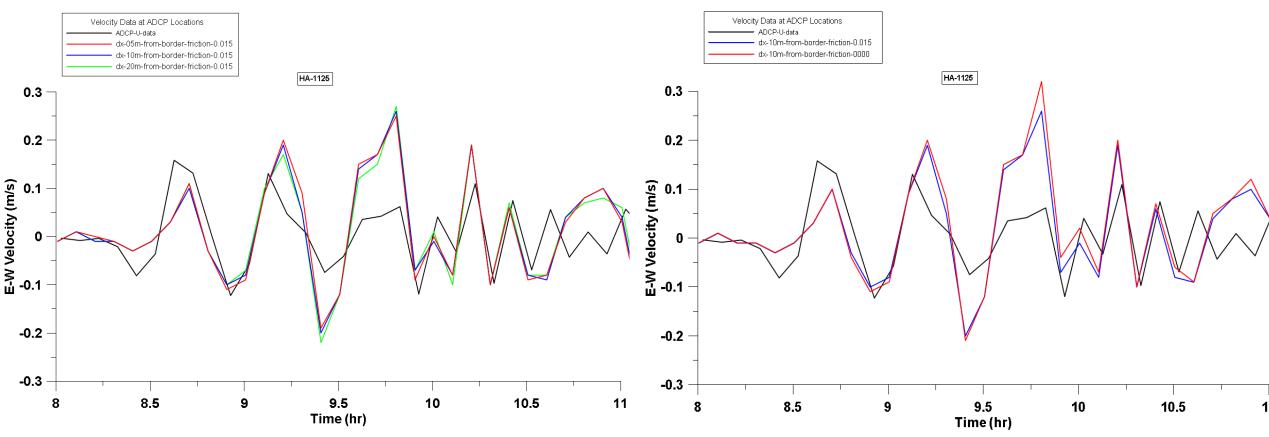


HA1125: Approach to Hilo Harbor, Comparisons of the E-W current speeds for three different resolutions (Manning's Coefficient is 0.015)

HA1125: Approach to Hilo Harbor, Comparisons of the E-W current speeds for 10m resolution with Manning's coefficient 0 and 0.015

BM#2 Plot frequency 360 sec (field data), 2.5 sec (numerical data)

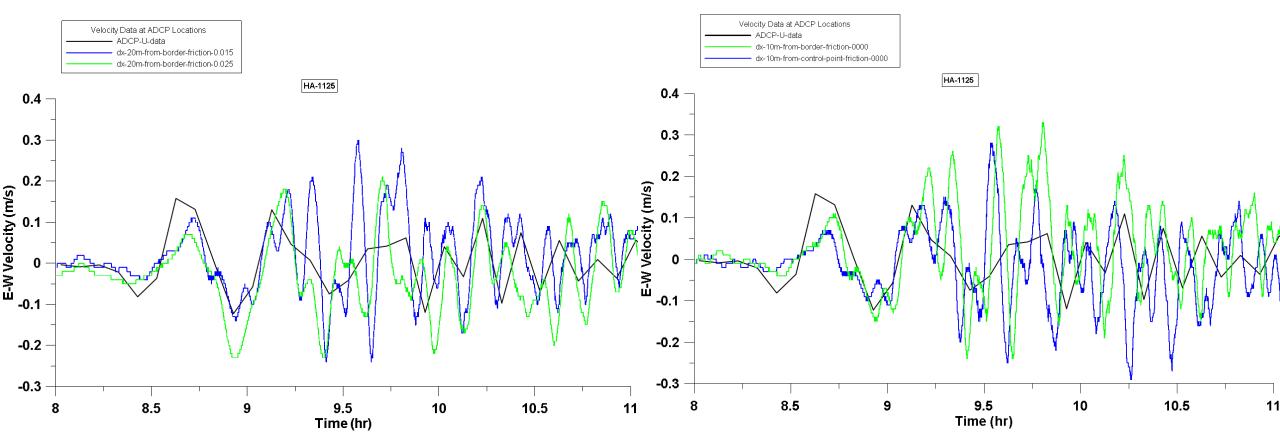
Current Speeds in E-W Direction



HA1125: Approach to Hilo Harbor, Comparisons of the E-W current speeds for three different resolutions (Manning's Coefficient is 0.015)

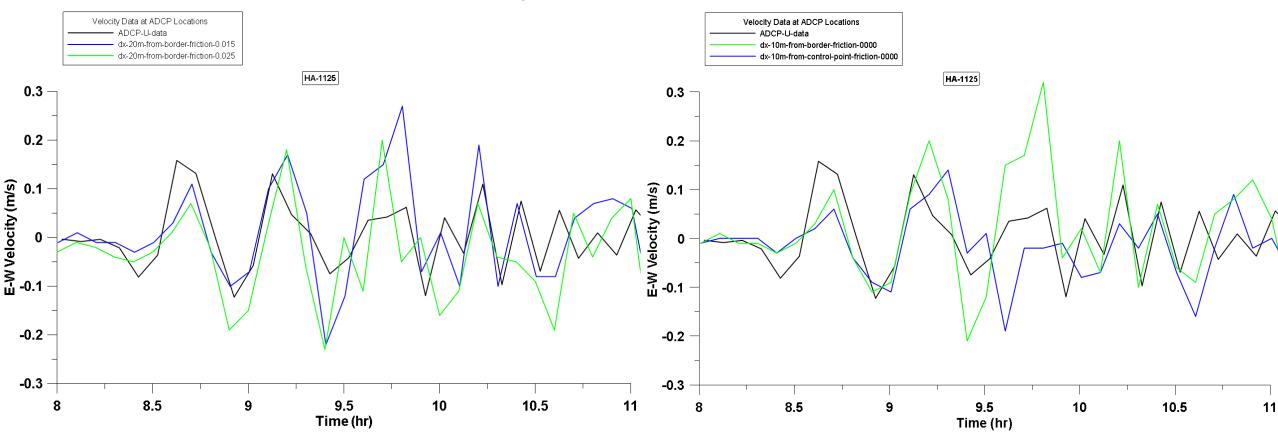
BM#2 Data plot frequency 360 sec

HA1125: Approach to Hilo Harbor, Comparisons of the E-W current speeds for 10m resolution with Manning's coefficient 0 and 0.015



HA1125: Approach to Hilo Harbor, Comparisons of the E-W current speeds for 20m resolution with Manning's coefficient 0.015 and 0.025

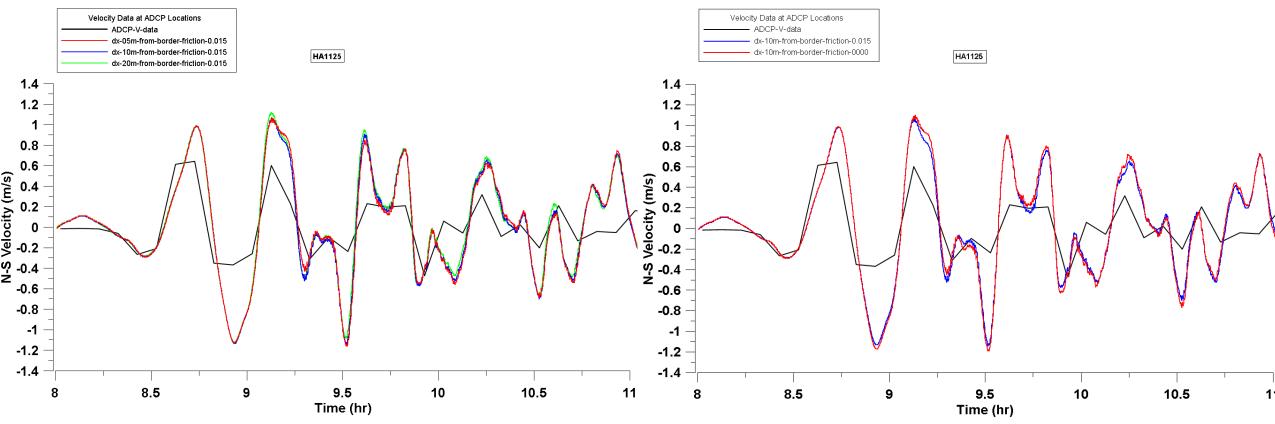
HA1125: Approach to Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in E-W direction



HA1125: Approach to Hilo Harbor, Comparisons of the E-W current speeds for 20m resolution with Manning's coefficient 0.015 and 0.025

HA1125: Approach to Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in E-W direction

Current Speeds in N-S Direction



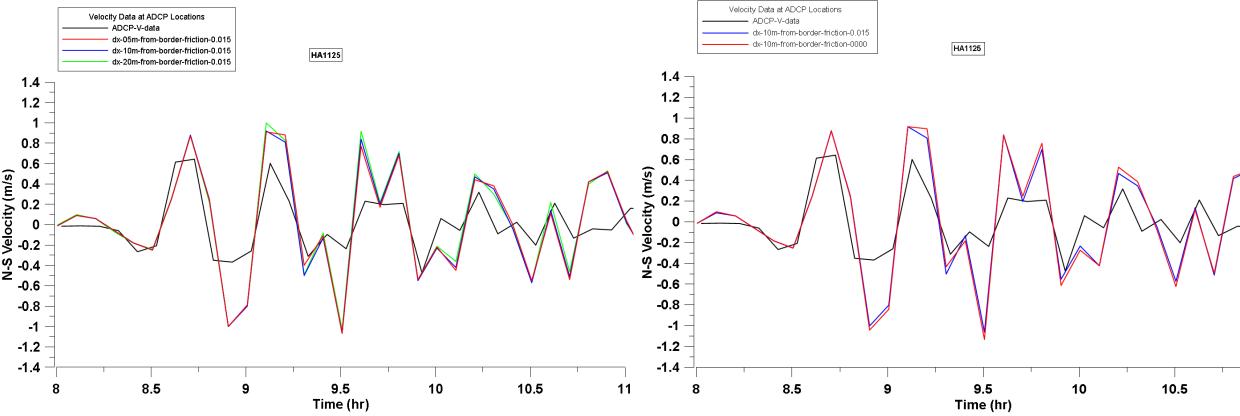
HA1125: Approach to Hilo Harbor, Comparisons of the N-S current speeds for three different resolutions (Manning's Coefficient is 0.015)

HA1125: Approach to Hilo Harbor, Comparisons of the N-S current speeds for 10m resolution with Manning's coefficients 0 and 0.015

BM#2

Plot frequency 360 sec (field data), 2.5 sec (numerical data)

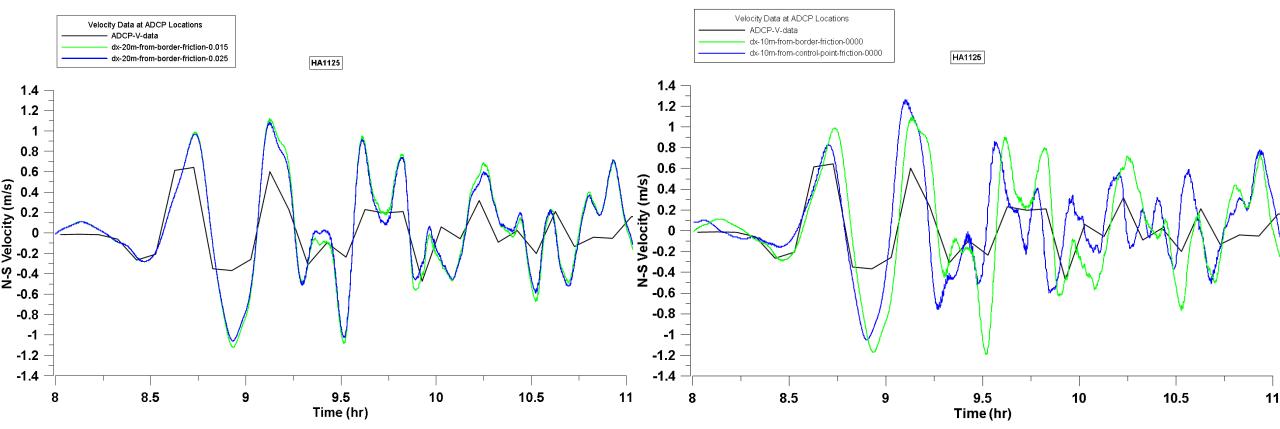
Current Speeds in N-S Direction



HA1125: Approach to Hilo Harbor, Comparisons of the N-S current speeds for three different resolutions (Manning's Coefficient is 0.015)

HA1125: Approach to Hilo Harbor, Comparisons of the N-S current speeds for 10m resolution with Manning's coefficients 0 and 0.015

Current Speeds in N-S Direction



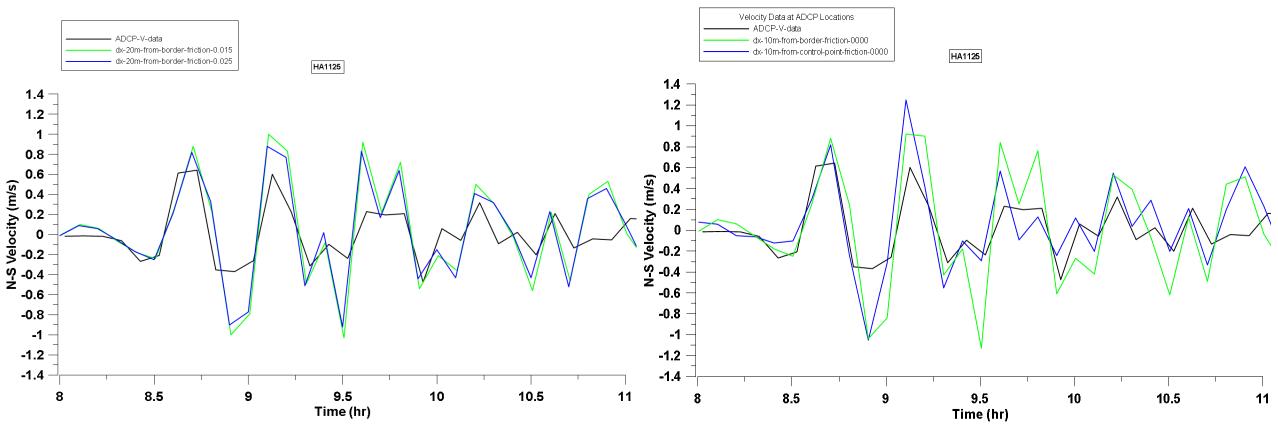
HA1125: Approach to Hilo Harbor, Comparisons of the N-S current speeds for 20m resolution with Manning's coefficients 0.015 and 0.025

HA1125: Approach to Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in N-S direction

BM#2

Plot frequency 360 sec (field data), 2.5 sec (numerical data)

Current Speeds in N-S Direction



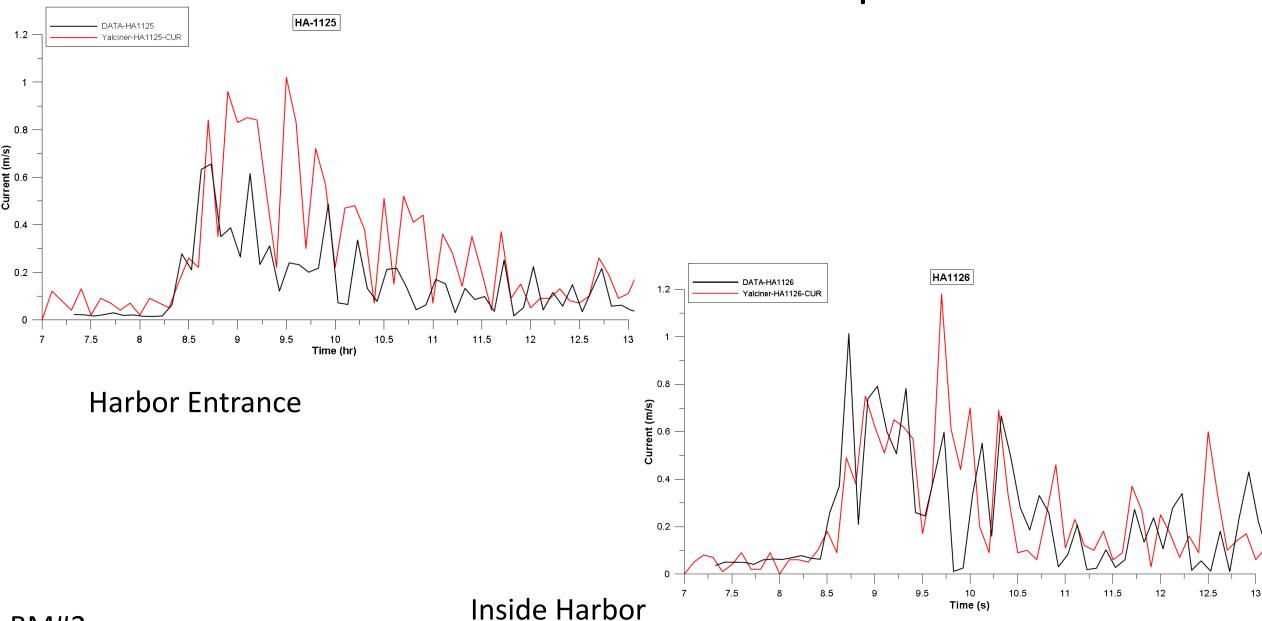
HA1125: Approach to Hilo Harbor, Comparisons of the N-S current speeds for 20m resolution with Manning's coefficients 0.015 and 0.025

HA1125: Approach to Hilo Harbor, Comparison of the input location of incident wave with 10m resolution for current speeds in N-S direction

BM#2

Data plot frequency 360 sec

COMPARISON OF RESULTS - Maximum Speeds





NTHMP - Mapping & Modeling Benchmarking Workshop: Tsunami Currents

Benchmark Problem #3
Japan 2011 tsunami in Tauranga Harbor, New Zealand

Ahmet C. Yalciner, Andrey Zaytsev, Utku Kanoglu

Project Assistant, Rozita Kian

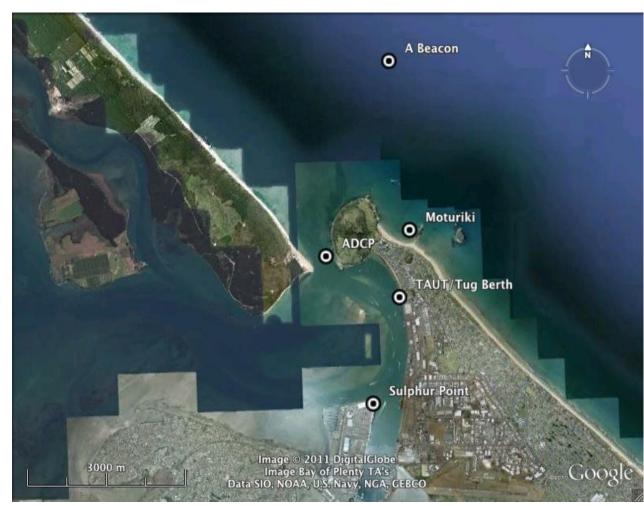
Middle East Technical University

Department of Civil Engineering Department

Department of Engineering Sciences

BM#3 – Japan 2011 Tsunami in Tauranga, New Zealand

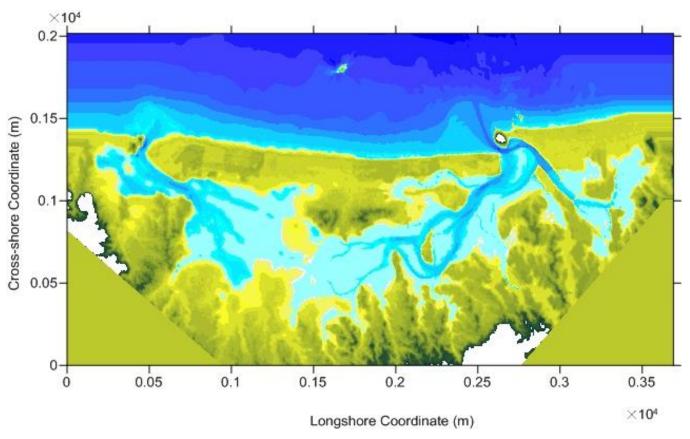
- The unique component of this benchmark test is to attempt to include the effects of the tides and compare free surface elevation (from tide stations) and velocity information (from and ADCP).
- The results obtained from these simulation studies using NAMI DANCE and the comparisons with the actual data for free surface elevation and current speeds are provided.



BM#3 – Japan 2011 Tsunami in Tauranga Harbor, New Zealand **Bathymetry**

• Bathymetry data is provided for 30m resolution (1 arcsec grid).

- Maximum Water Depth: 37.6m
- Depth of Input wave in ABeacon: 25m
- Simulation time step: 0.25 sec
- Manning Coeff: 0.025



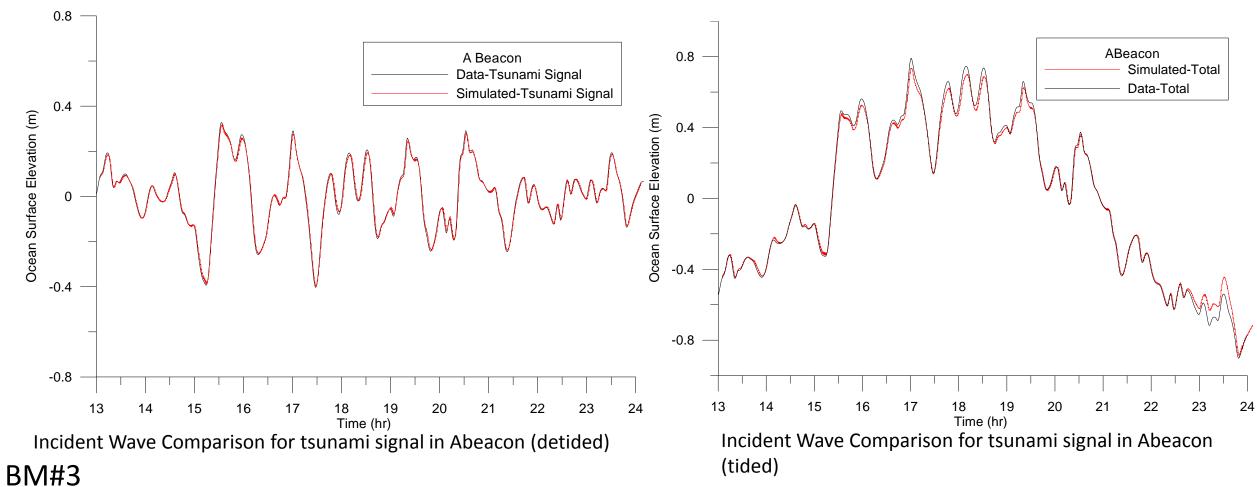
Bathymetry used in simulations for 30 m resolution

BM#3 – Japan 2011 Tsunami in Tauranga Harbor, New Zealand Gauge Points

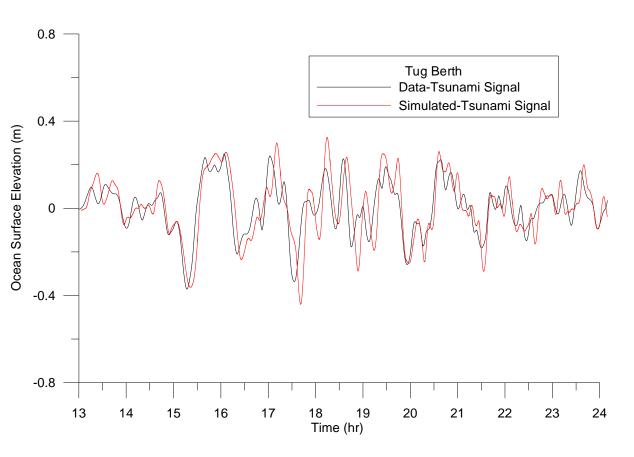
Gauge Point	X Coordinate (m)	Y Coordinate (m)	
ABeacon	2.724e4	1.846e4	
Tug Berth	3.085e4	1.512e4	
Sulfur	3.2e4	1.347e4	
Moturiki	3.005e4	1.61e4	

BM#3 – Japan 2011 Tsunami in Tauranga Harbor, New Zealand Incident Wave

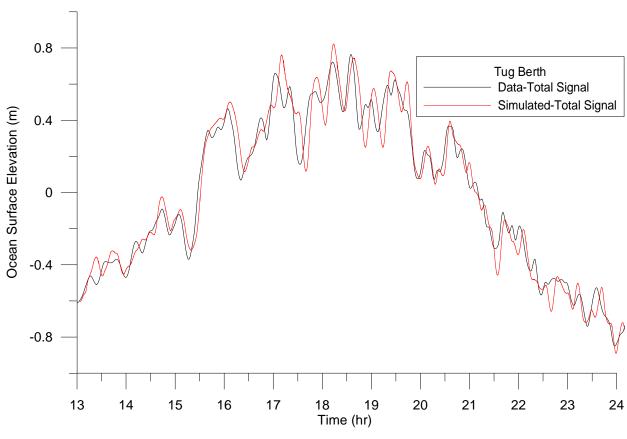
• Since the incident wave data is given with 60 second (1 minute) time intervals, the data is obtained again for 0.25 second intervals for 30m resolution by making linear interpolation.



COMPARISON OF RESULTS – Water Surface Elevation @ Tug Berth

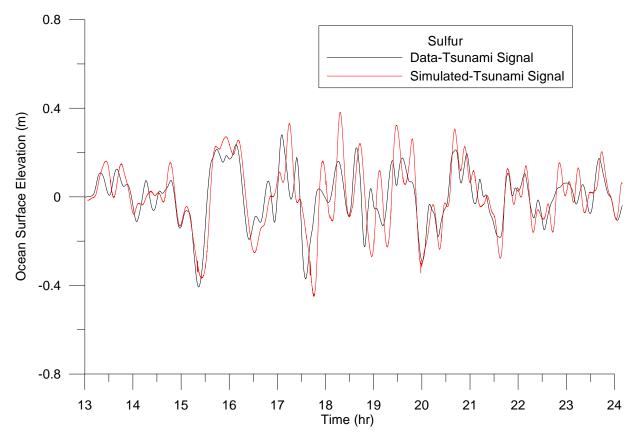


Comparison of water surface elevation at Tug Berth for tsunami signal (detided)

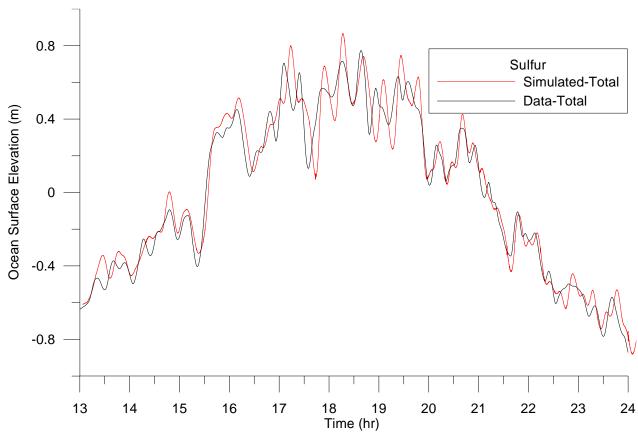


Comparison of water surface elevation at Tug Berth for total signal (tided)

COMPARISON OF RESULTS – Water Surface Elevation @ Sulfur

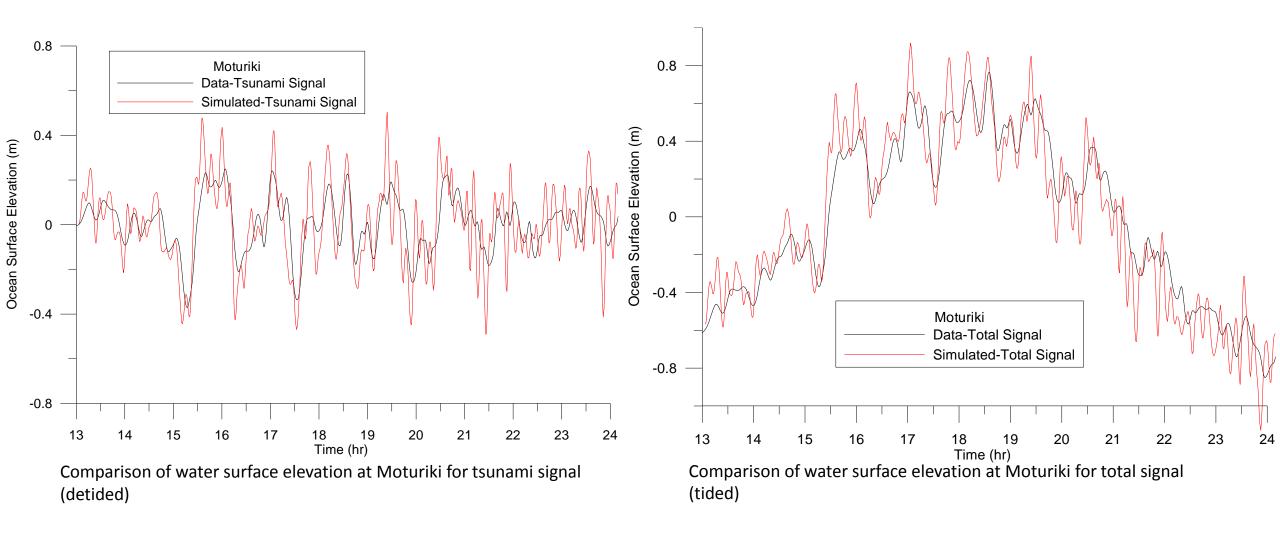


Comparison of water surface elevation at Sulfur for tsunami signal (detided)

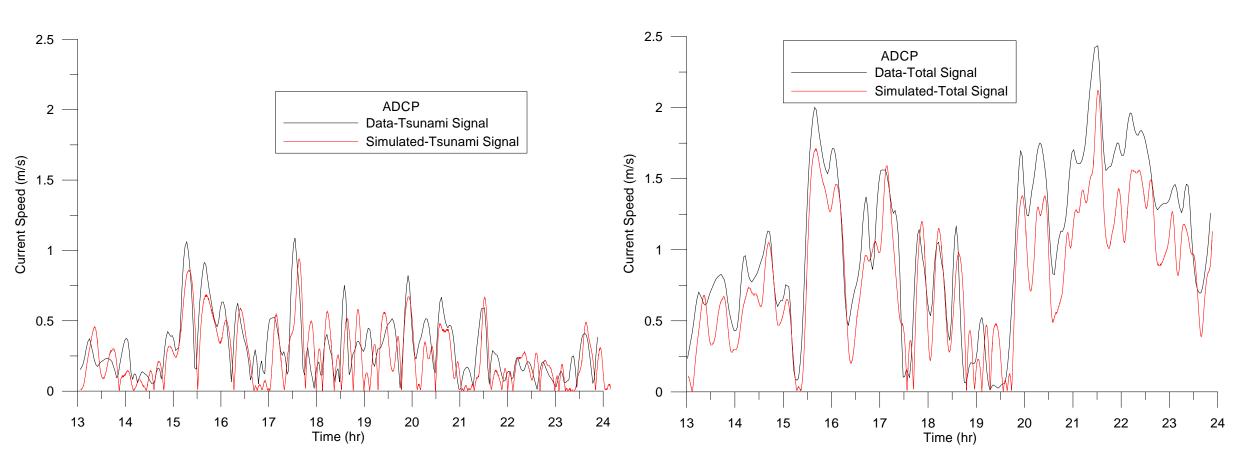


Comparison of water surface elevation at Sulfur for total signal (tided)

COMPARISON OF RESULTS – Water Surface Elevation @ Moturiki



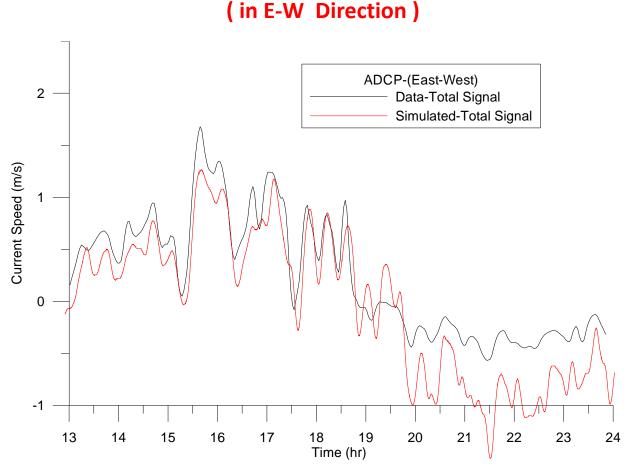
COMPARISON OF RESULTS – Current Speeds@ ADCP



Comparison of current speed at ADCP for tsunami signal (detided)

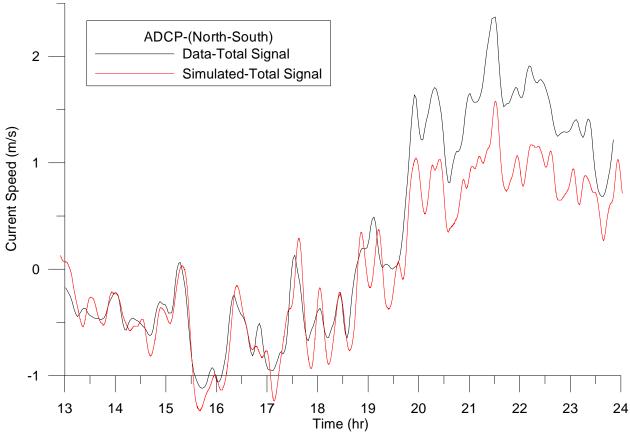
Comparison of current speed at ADCP for tsunami signal (tided)

COMPARISON OF RESULTS – Current Speeds@ ADCP (Total Signal-Tided)



Comparison of current speed in E-W direction at ADCP for totall signal (tided)

(in N-S Direction)



Comparison of current speed in N-S direction at ADCP for total signal (tided)



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Benchmark Problem #4
Single long-period wave propagating onto a small-scale model of the town of Seaside, Oregon

Ahmet C. Yalciner, Andrey Zaytsev, Utku Kanoglu Project Assistants, Rozita Kian, Naeimeh Shaghrivand

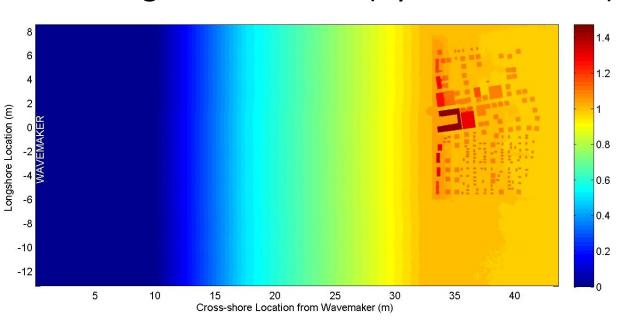
Middle East Technical University

Department of Civil Engineering Department

Department of Engineering Sciences

BM#4 – Bathymetry, Gauges

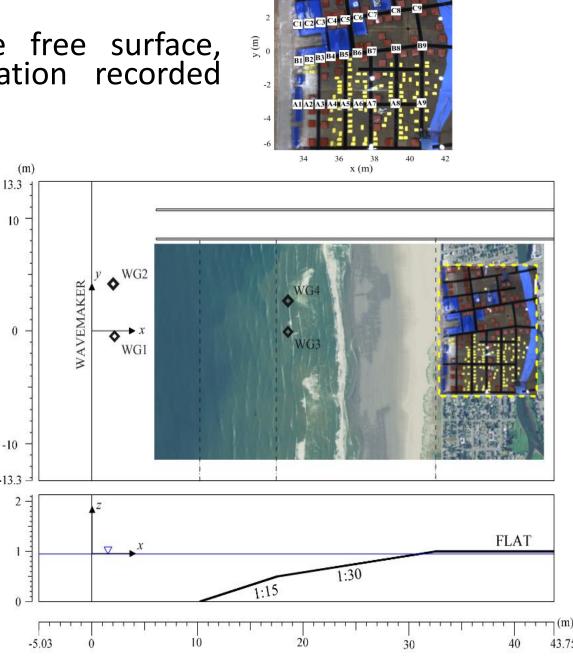
• For this benchmark, we will compare free surface, velocity, and momentum flux information recorded throughout the tank. (By NAMI DANCE)



resolution: 0.1 m

Maximum Water Depth: 0.97m Simulation time step: 0.0005 sec

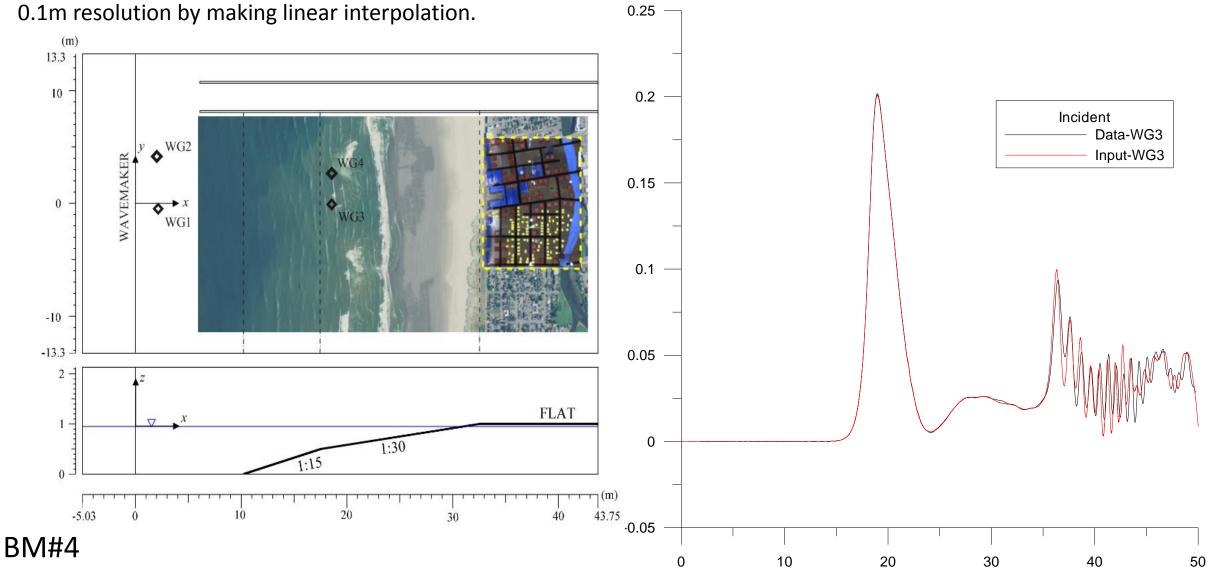
Manning Coeff: 0.01



BM#4 – Incident Wave @ WG3

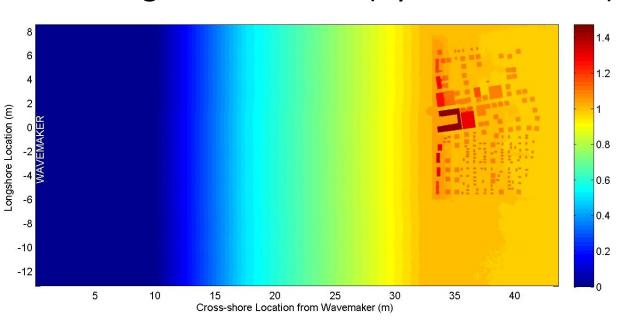
Since the incident wave data is given with 0.02 second time intervals, the data is obtained again for 0.0005 second intervals for 0.1m resolution by making linear interpolation.

0.25 \neg



BM#4 – Bathymetry, Gauges

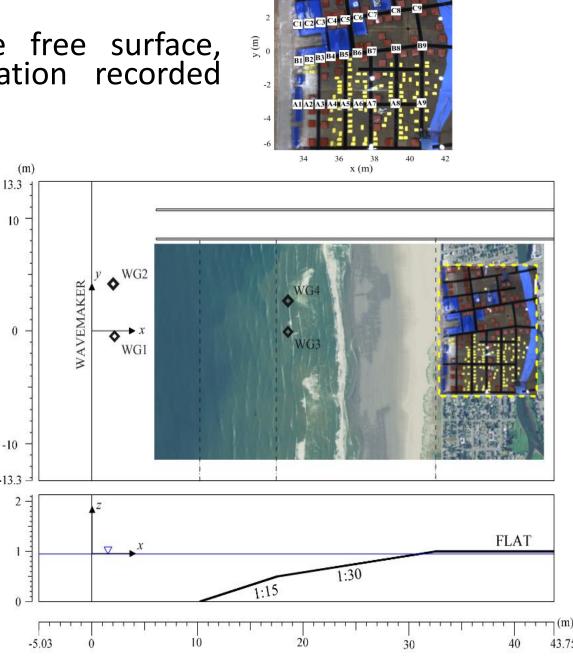
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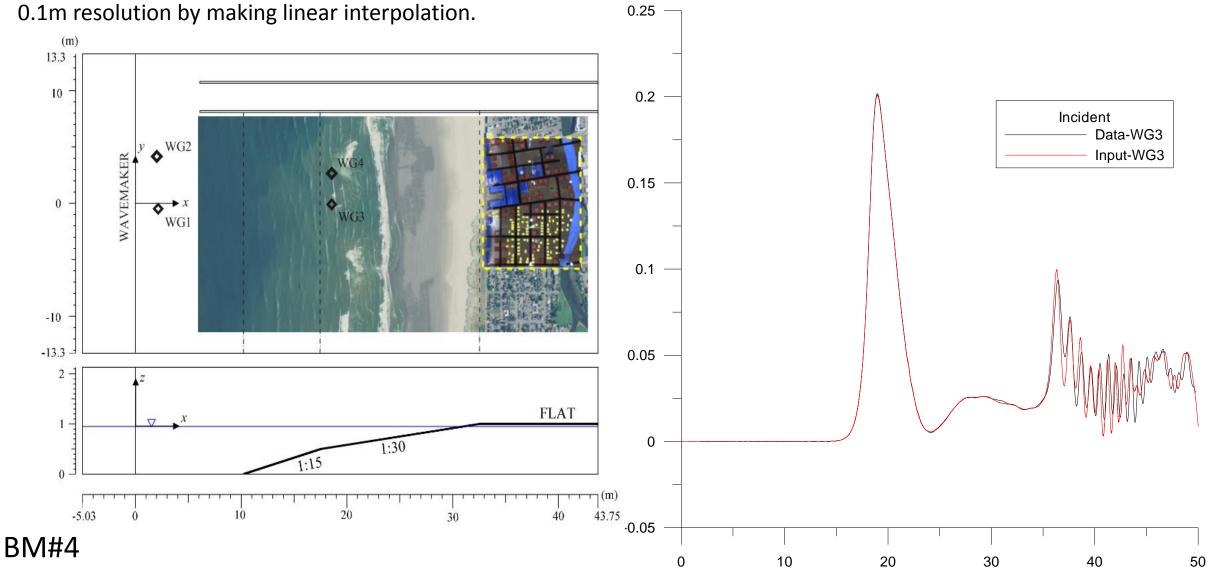
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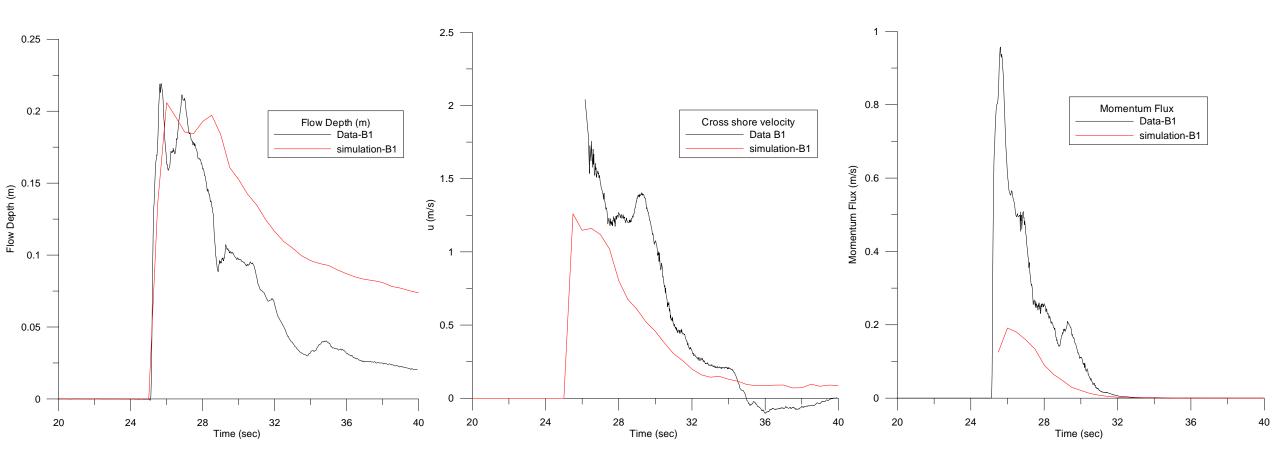
BM#4 – Incident Wave @ WG3

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0.25 \neg



COMPARISON OF RESULTS – @ (B1) Location

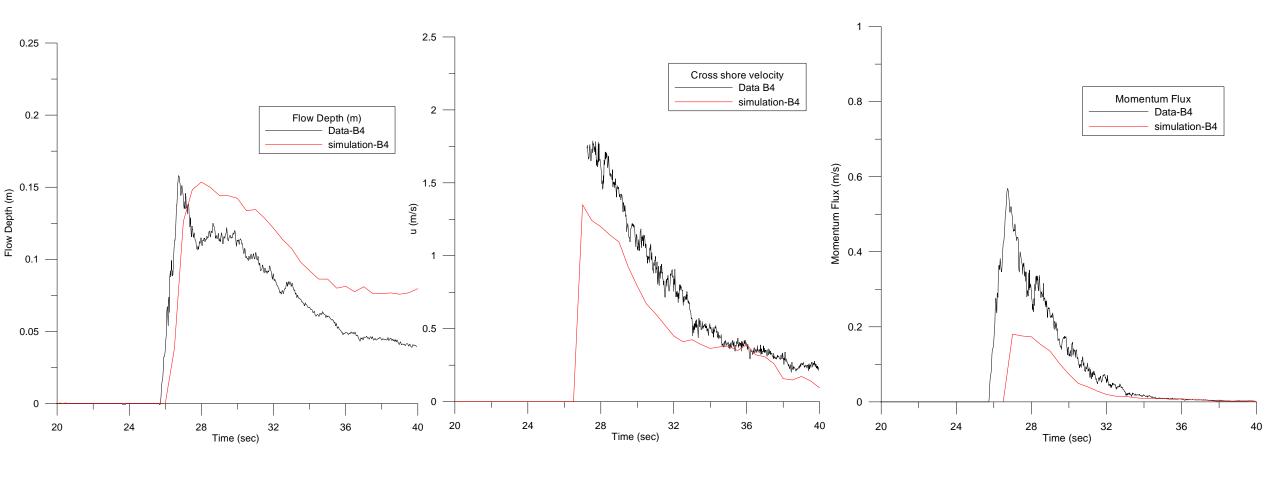


Comparison of flow depth

Comparison of cross shore velocity

Comparison of momentum flux

COMPARISON OF RESULTS – @ (B4) Location

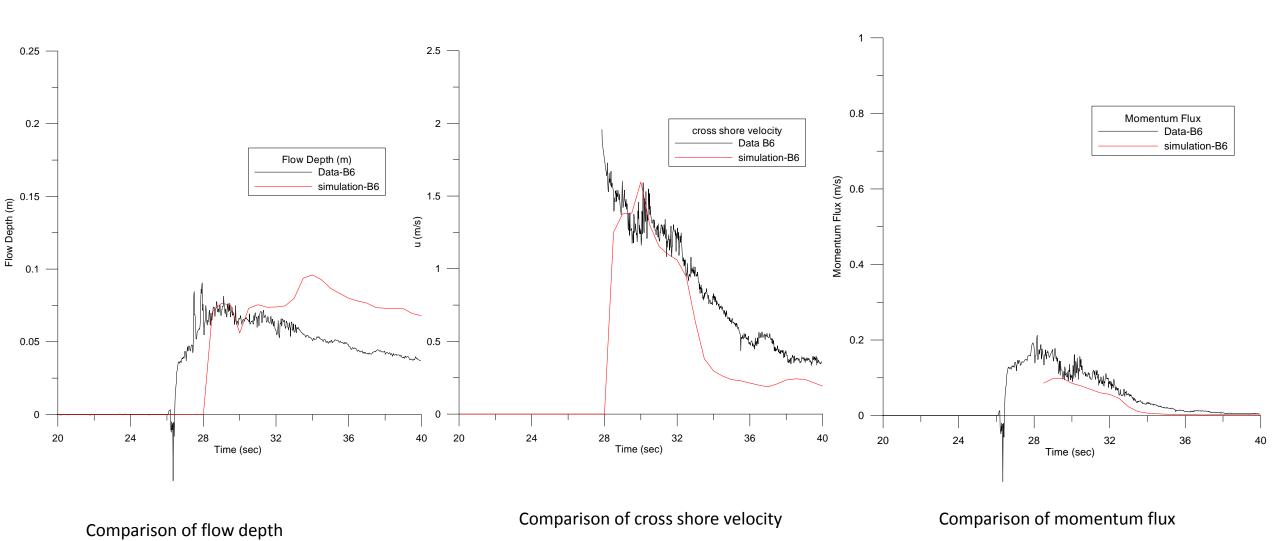


Comparison of flow depth

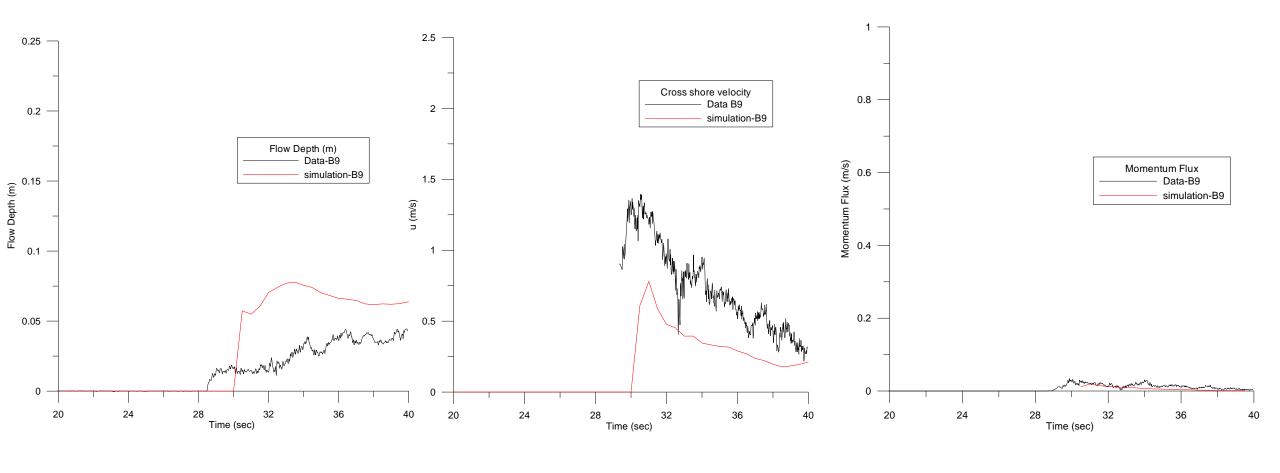
Comparison of cross shore velocity

Comparison of momentum flux

COMPARISON OF RESULTS – @ (B6) Location



COMPARISON OF RESULTS – @ (B9) Location



Comparison of flow depth

Comparison of cross shore velocity

Comparison of momentum flux



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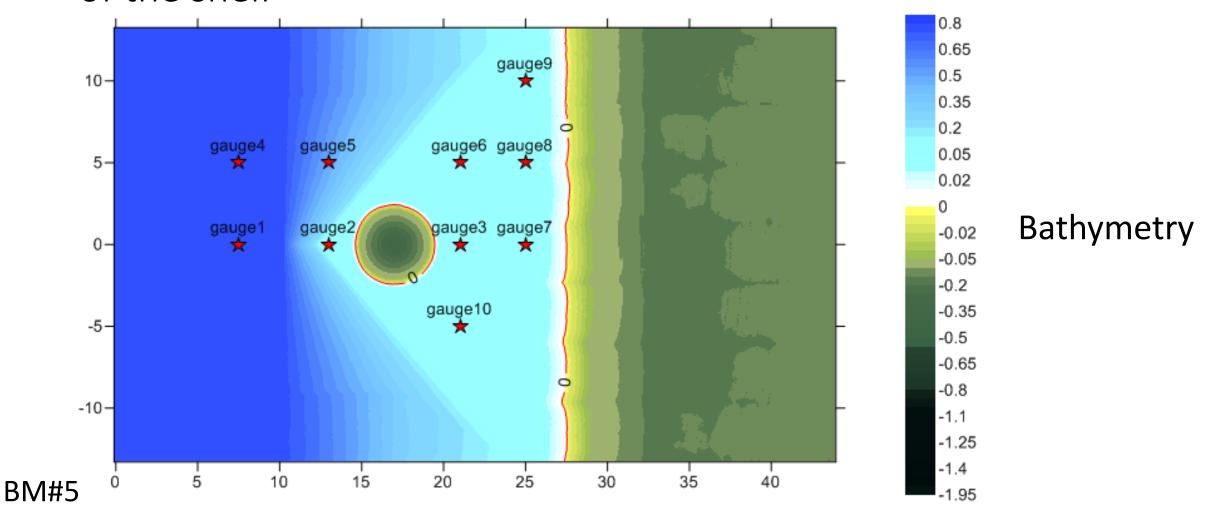
Benchmark Problem #5
Experiment on a single solitary wave propagating up a triangular shaped shelf with an island feature located at the offshore point of the shelf

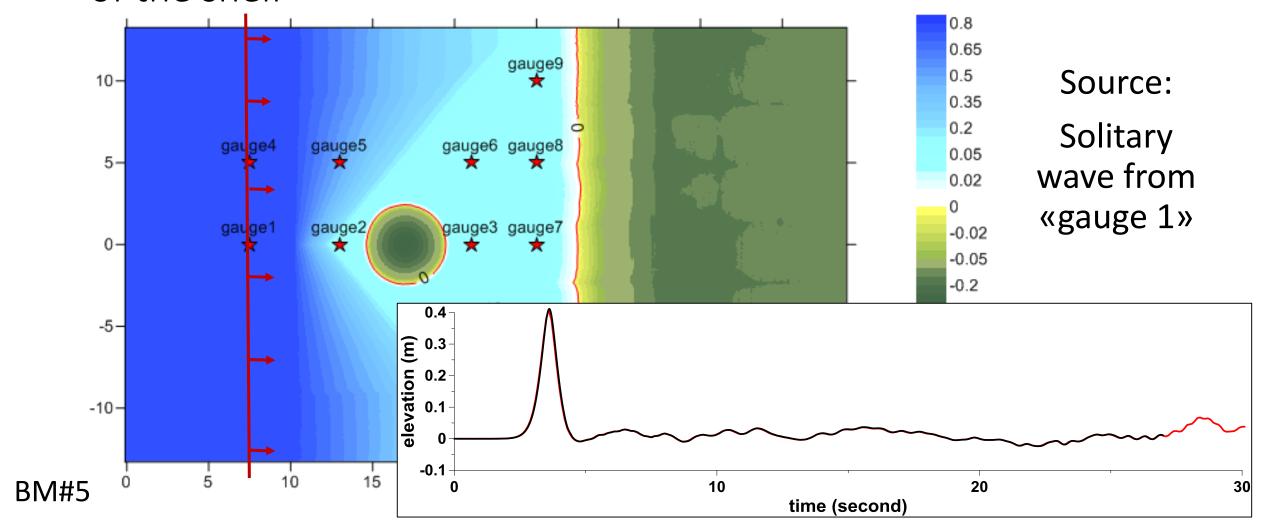
Ahmet C. Yalciner, Andrey Zaytsev, Utku Kanoglu Research Assistant Betul Aytore

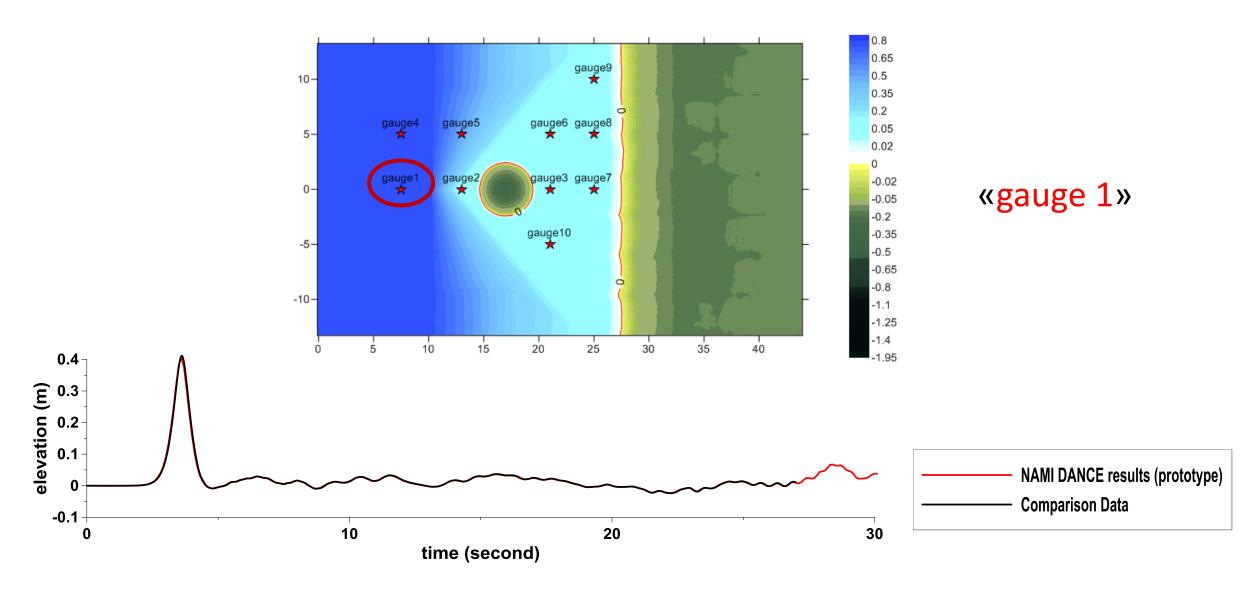
METU, Department of Civil Engineering and Department of Engineering Sciences

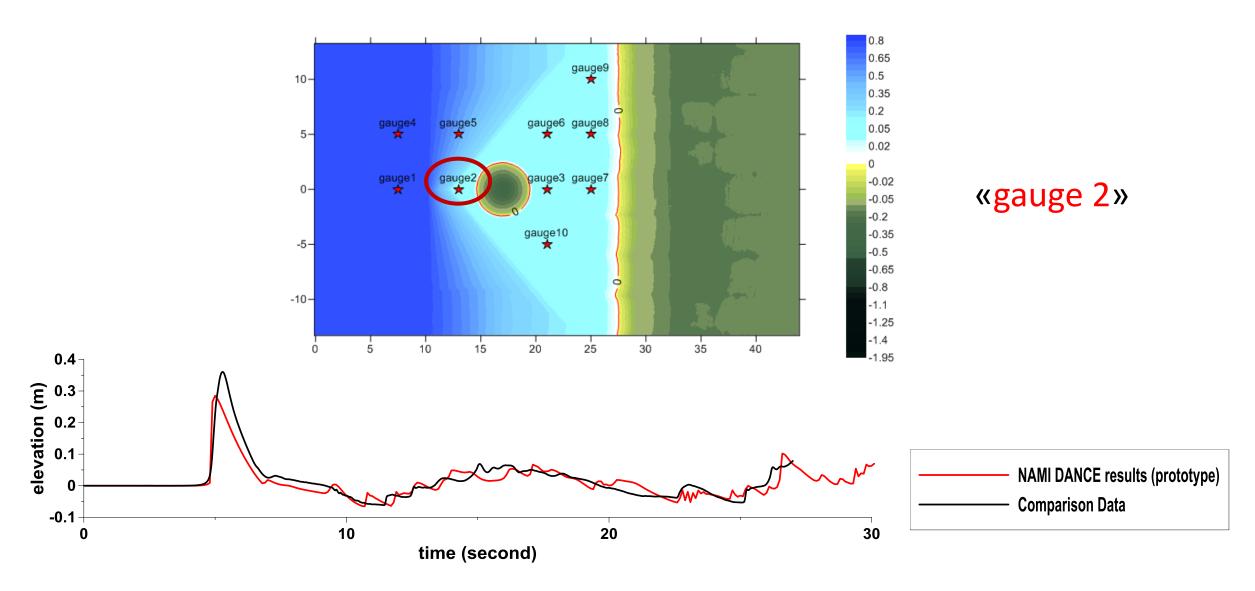
- Simulation studies are carried out by tsunami numerical modelling tool "NAMI DANCE" to understand the importance of model resolution and numerics on the prediction of tidal currents.
- NAMI DANCE is a computational tool developed by Profs Andrey Zaytsev, Ahmet Yalciner, Anton Chernov, Efim Pelinovsky and Andrey Kurkin as a collaboration for tsunami modeling.
- It provides direct simulation and efficient visualization of tsunamis to the user and for assessment, understanding and investigation of tsunami generation and propagation mechanisms. The model is tested and verified for research and operational purposes.

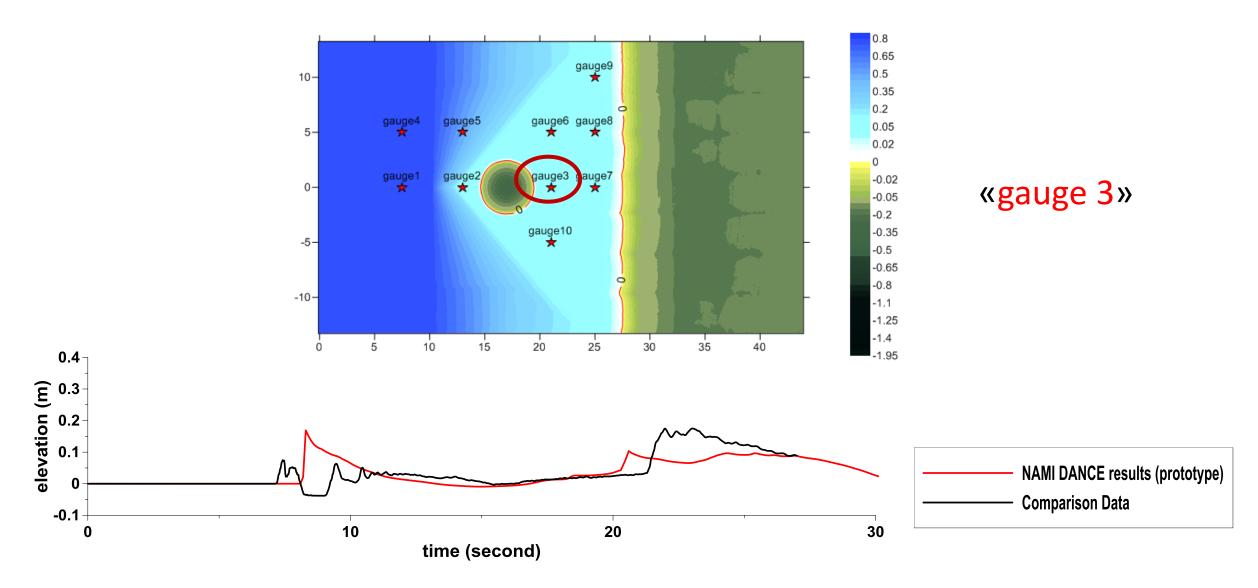
- This experiment has a single solitary wave propagating up a triangular shaped shelf with an island feature located at the offshore point of the shelf.
- Free surface information was recorded via resistance-type wave gauges and sonic wave gages. Velocity information was recorded via ADV's.
- For this benchmark, we will compare the free surface, velocity, and information recorded throughout the tank.

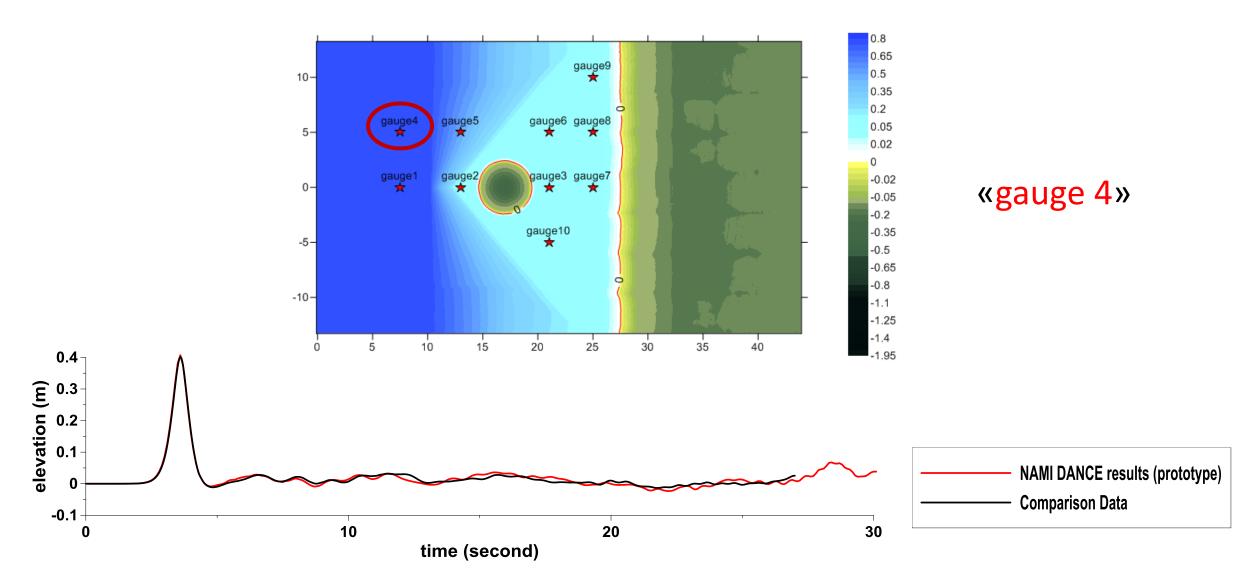


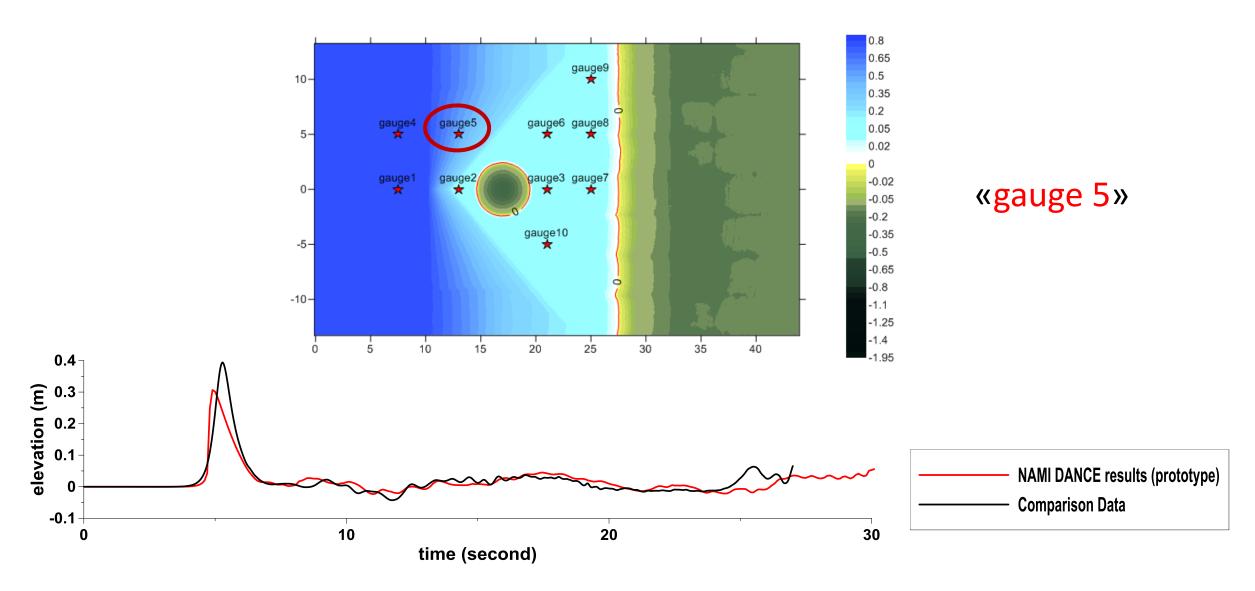


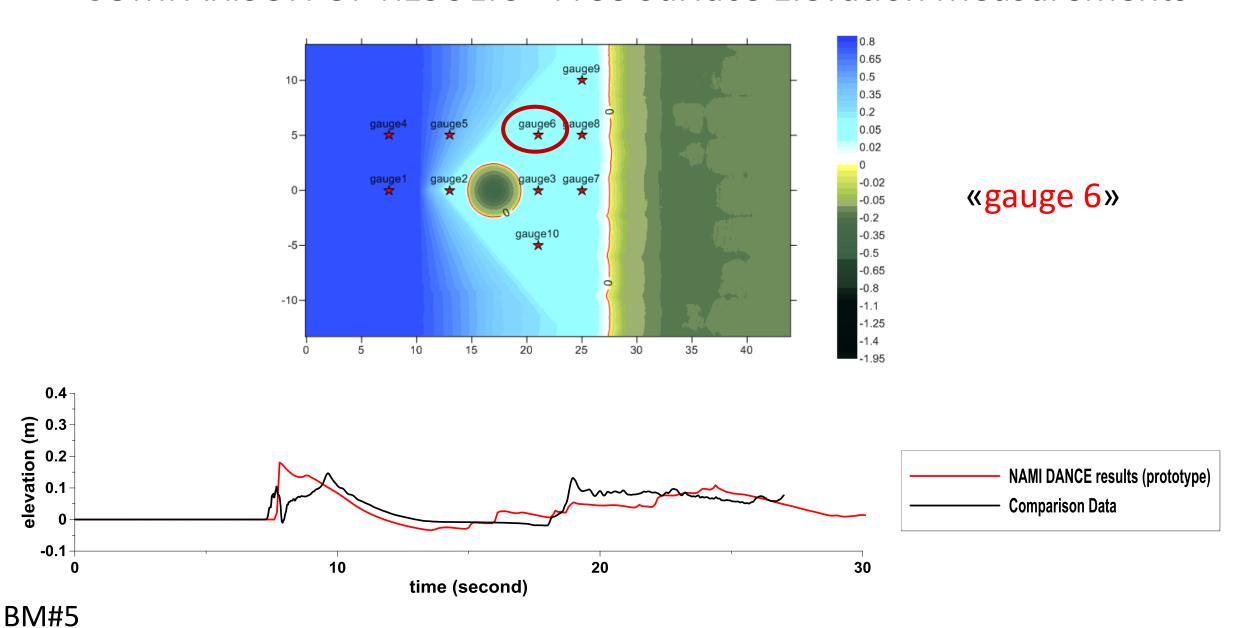


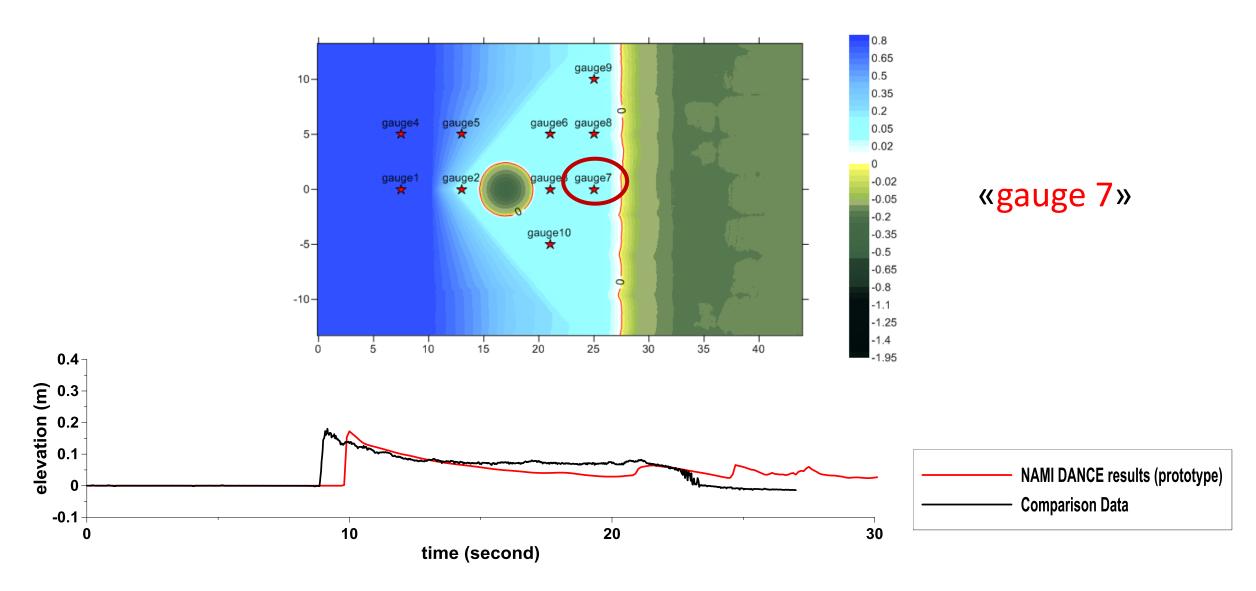


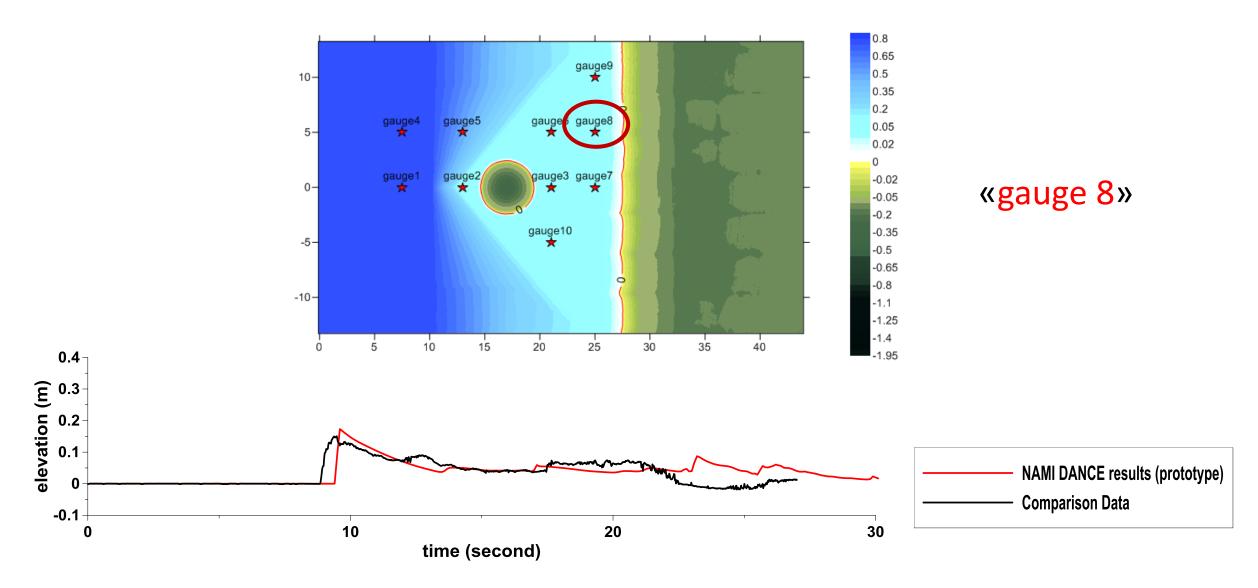


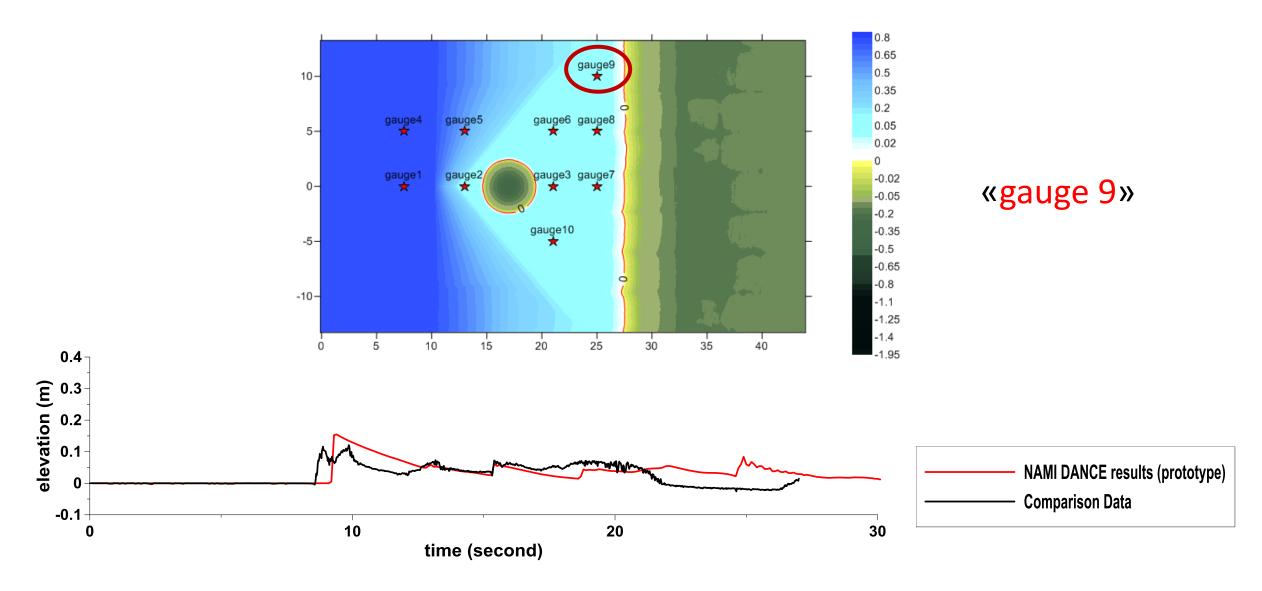




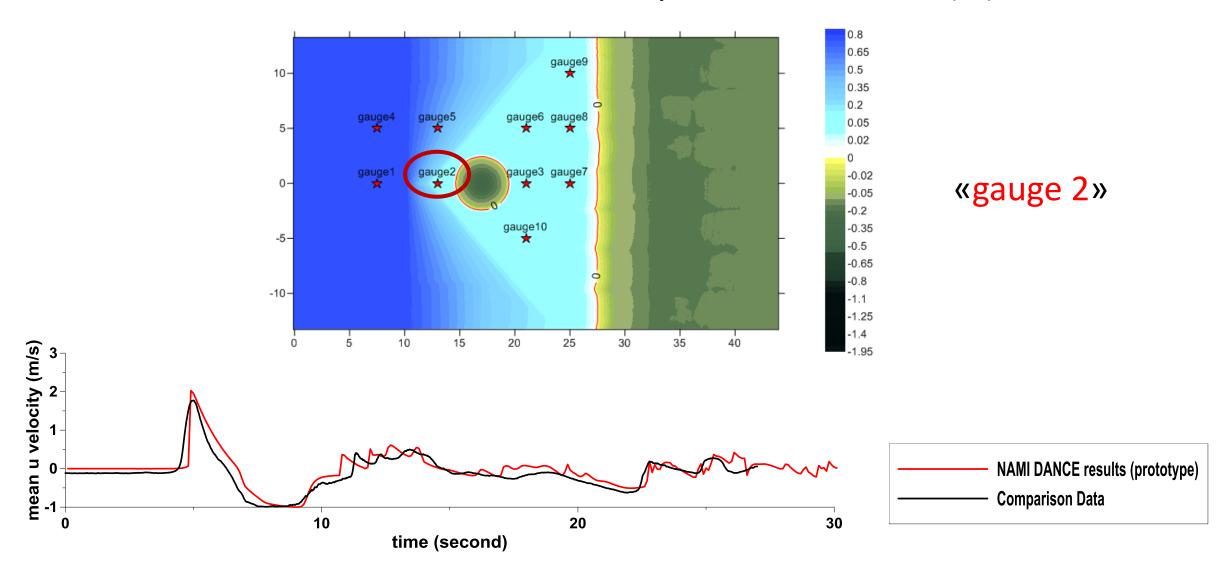




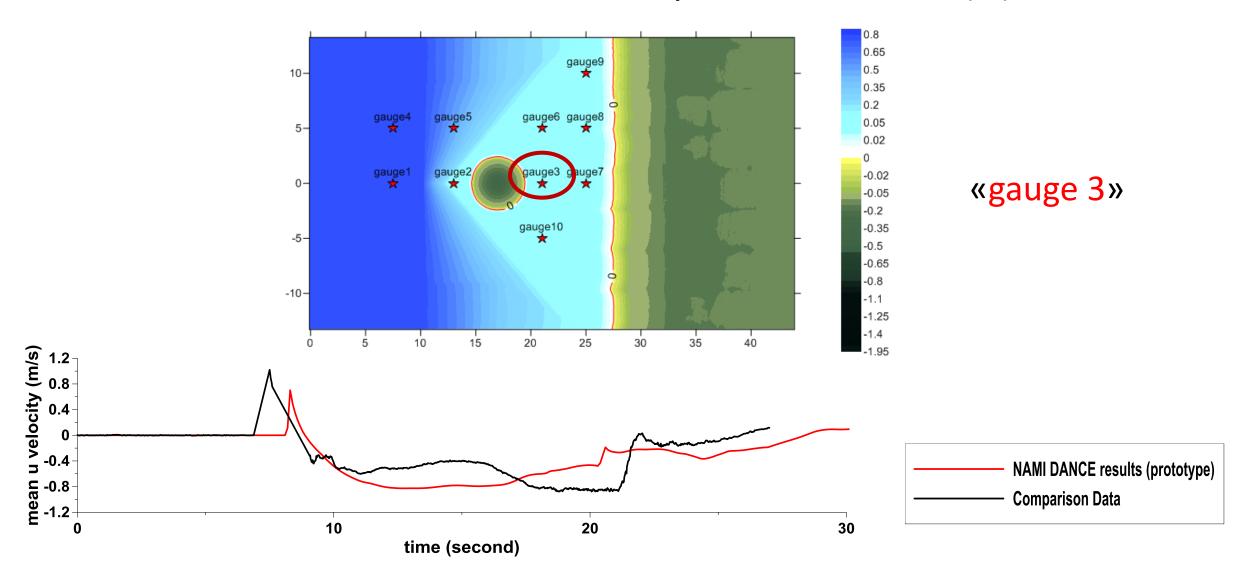




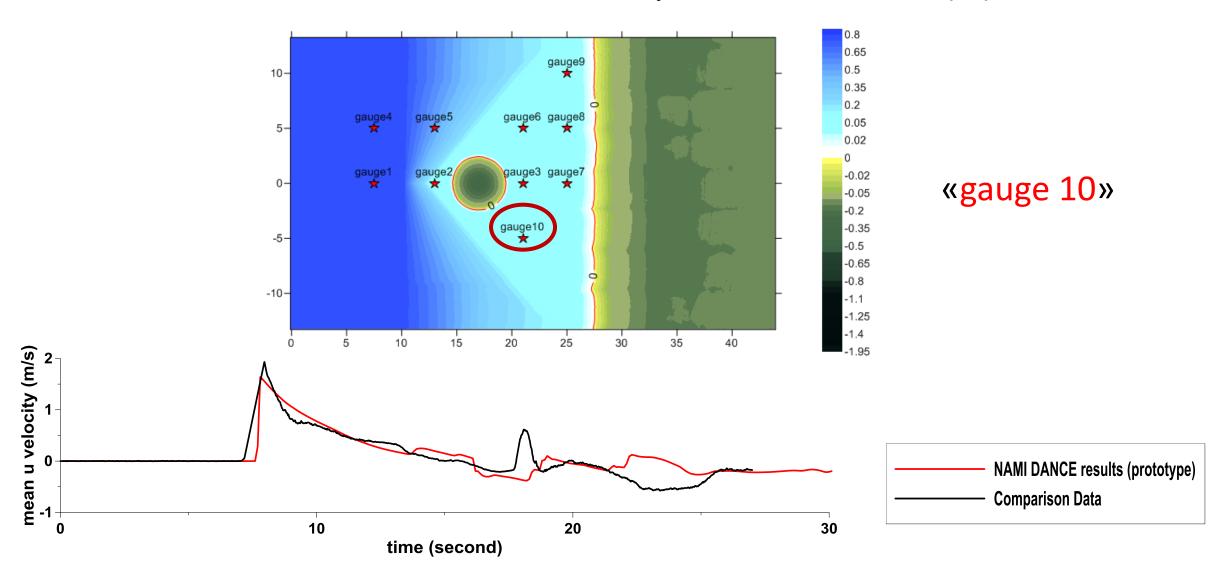
COMPARISON OF RESULTS - Velocity Measurements (U)



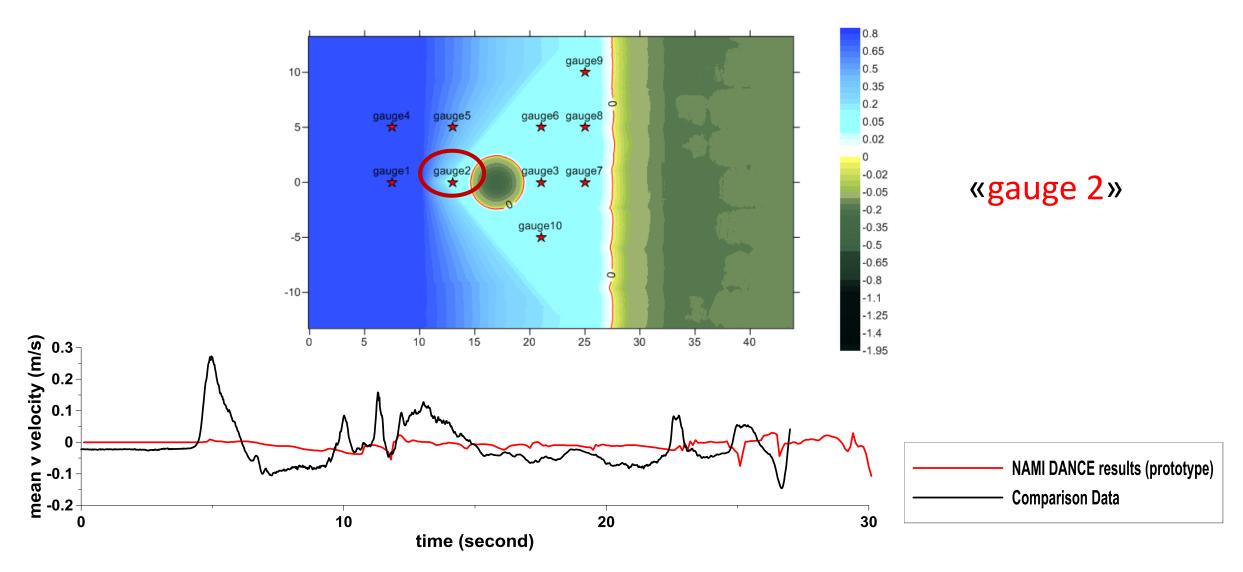
COMPARISON OF RESULTS - Velocity Measurements (U)



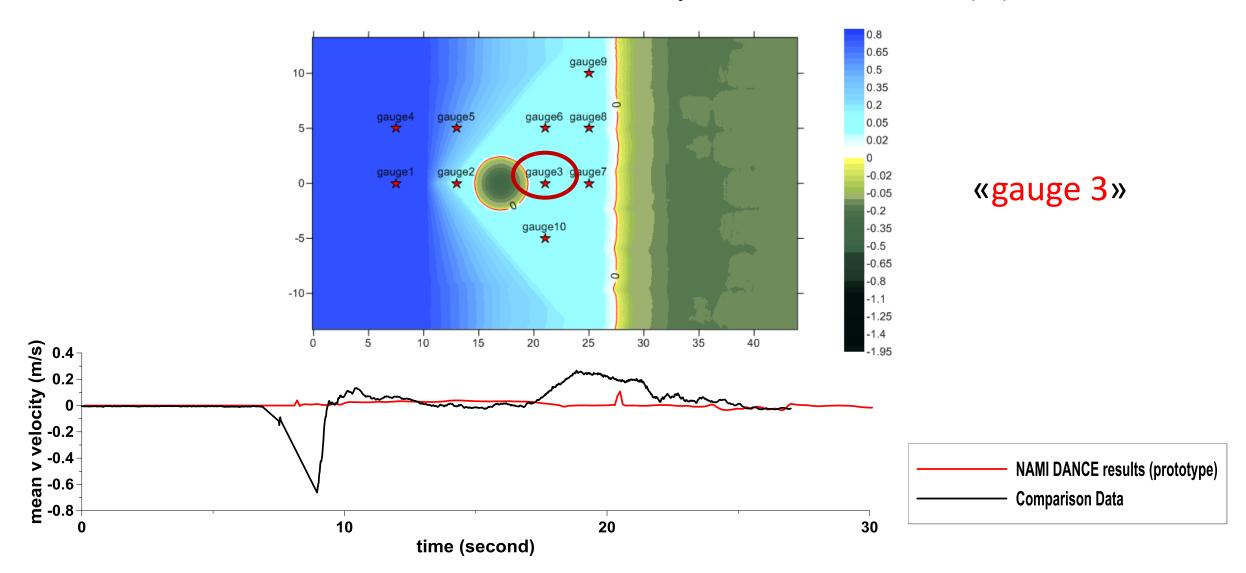
COMPARISON OF RESULTS - Velocity Measurements (U)



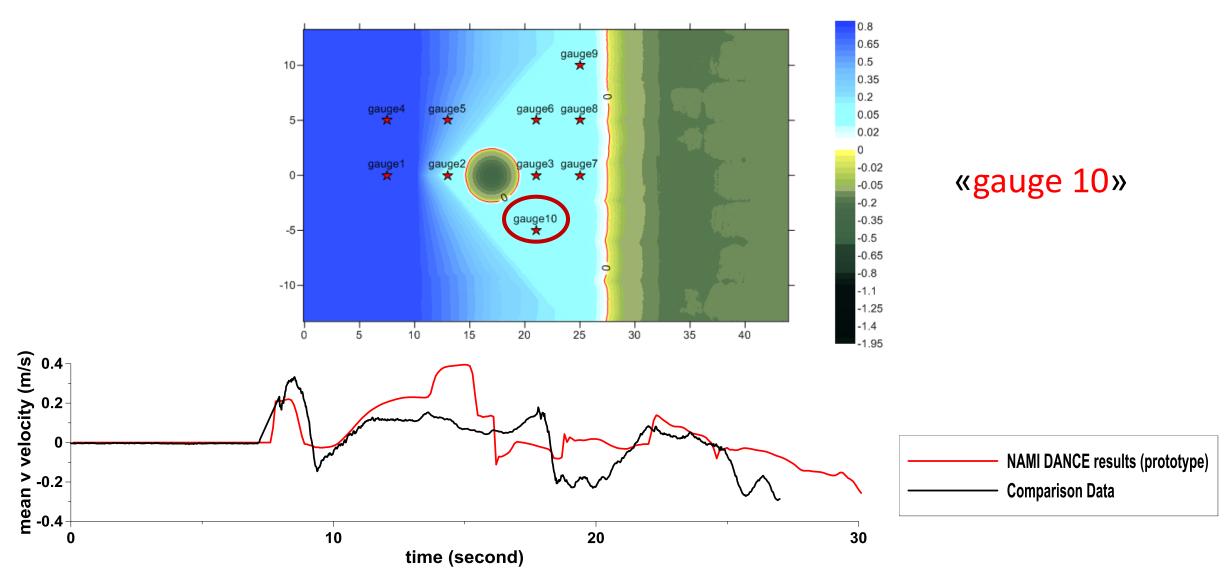
COMPARISON OF RESULTS - Velocity Measurements (V)



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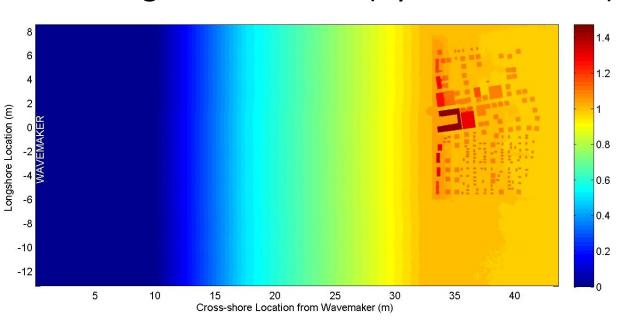
COMPARISON OF RESULTS - Velocity Measurements (V)



THANKS FOR YOUR ATTENTION

BM#4 – Bathymetry, Gauges

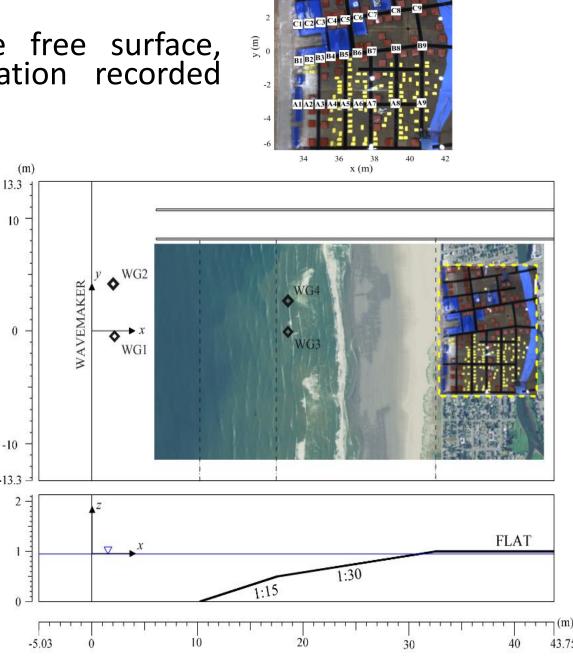
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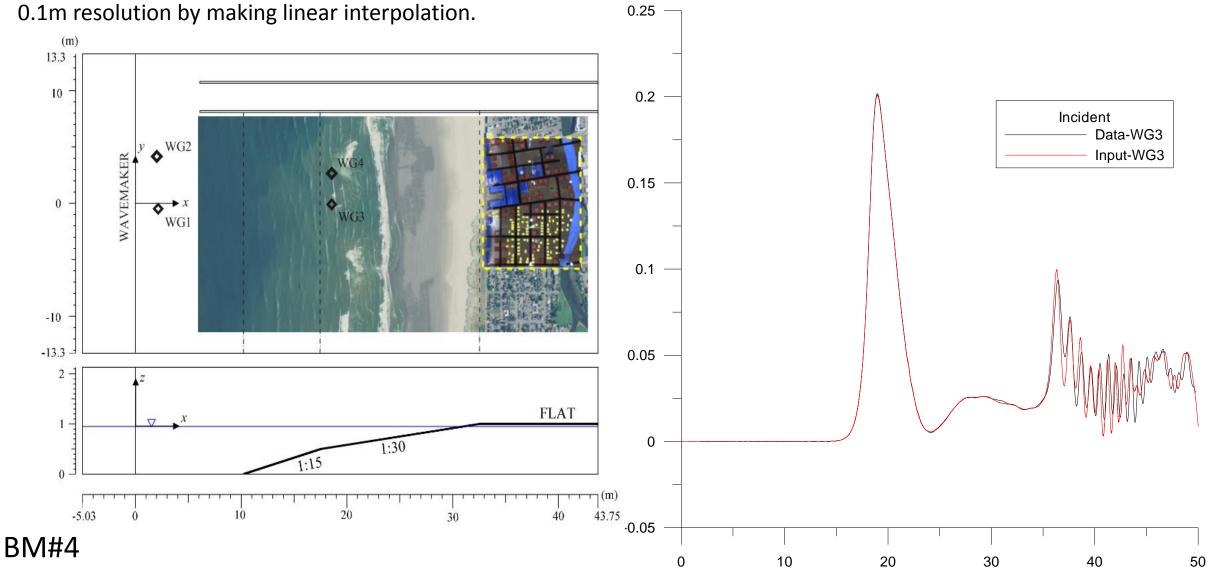
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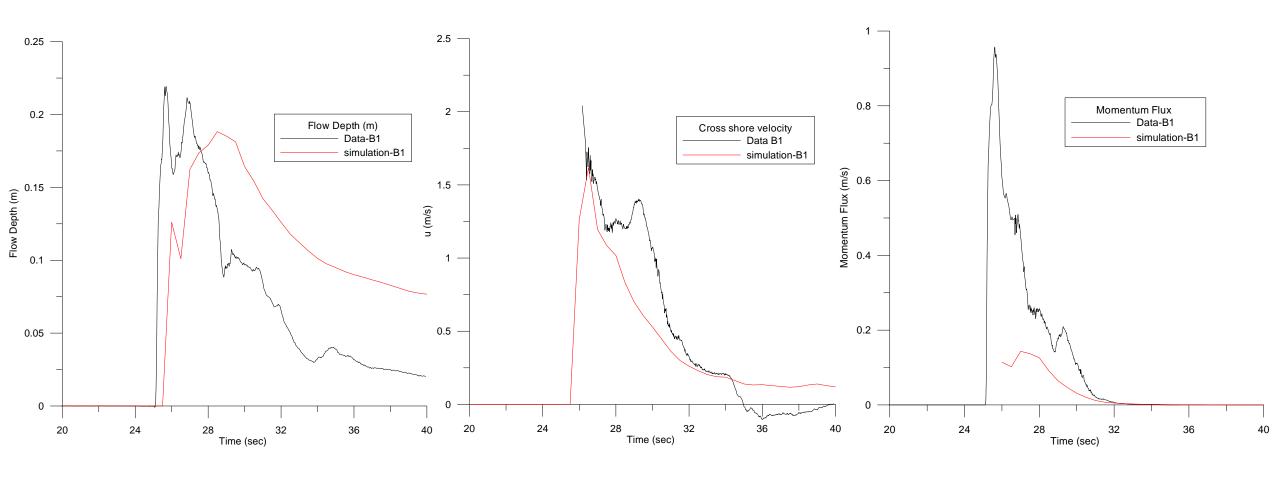
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0.25 \neg



COMPARISON OF RESULTS – @ (B1) Location

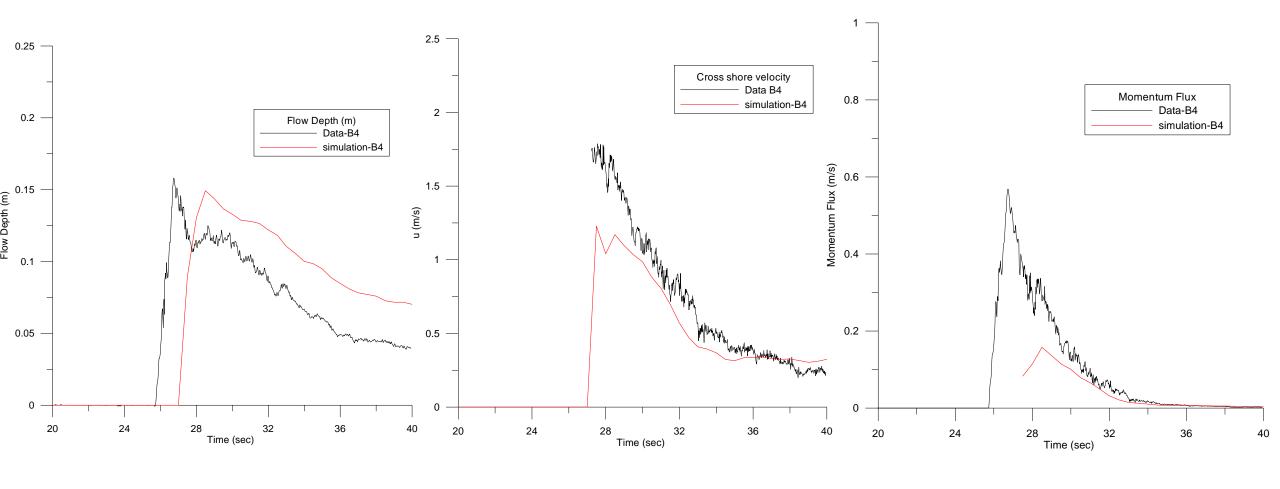


Comparison of flow depth

Comparison of cross shore velocity

Comparison of momentum flux

COMPARISON OF RESULTS – @ (B4) Location

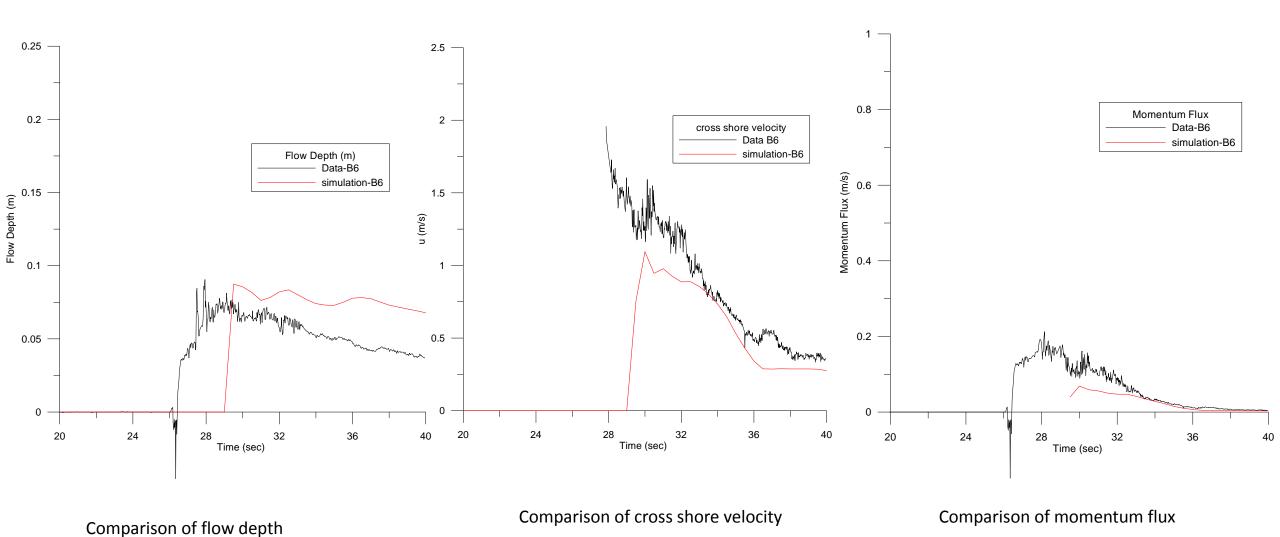


Comparison of flow depth

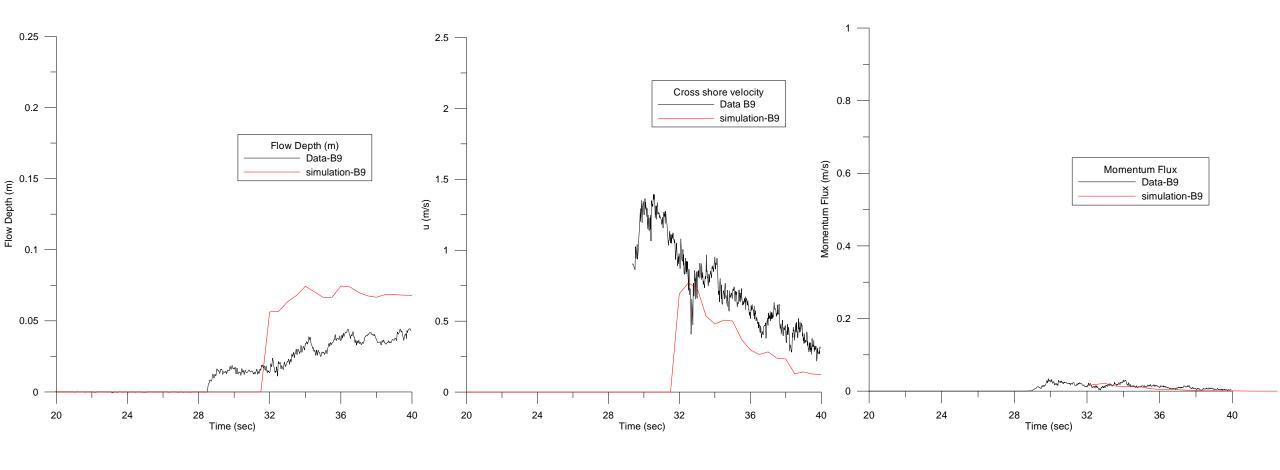
Comparison of cross shore velocity

Comparison of momentum flux

COMPARISON OF RESULTS – @ (B6) Location



COMPARISON OF RESULTS – @ (B9) Location



Comparison of cross shore velocity

Comparison of momentum flux

BM#4

Comparison of flow depth

THANKS FOR KIND ATTENTION